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FLOODPLAIN MANAGEMENT STUDY WILLOW CREEK WAUSHARA COUNTY, WISCONSIN



Prepared by:
United States Department of Agriculture
Soil Conservation Service
Madison, Wisconsin

In cooperation with:
Waushara County, Wisconsin
and the
Wisconsin Department of Natural Resources

April 1989

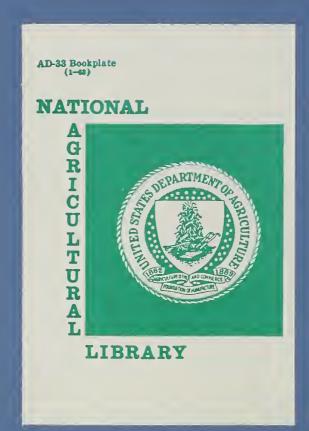
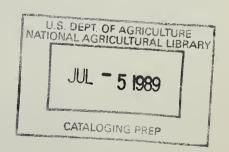


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Waushara County Floodplain Management Study Willow Creek

Introduction

The purpose of this study is to define the flood characteristics of Willow Creek from the Village of Auroraville to above the junction with the outlet of Bruce Creek. Waushara County requested the study through the Wisconsin Department of Natural Resources (DNR).

This report is prepared for use by the county in planning the use and regulation of the floodplain of Willow Creek.

The 100-year floodplain and the dam breach flood area have been delineated on the photomaps of Appendix A. The high water elevations and floodplain delineations are based on 5-year projected land use of the watershed, stream, floodplain, existing road crossings and a dam failure (breach) analysis.

The Soil Conservation Service carries out floodplain management studies in accordance with Federal Level Recommendation 3 of "A Unified National Program for Floodplain Management," and Section 6 of Public Law 83-566. The principles contained in Executive Order 11988, Floodplain Management, are addressed in this part.

In Wisconsin, the Soil Conservation Service coordinates floodplain management studies with the Wisconsin DNR, through a joint coordination agreement entered into in October 1978. The Wisconsin Water Resources Act (Chapter 614, Laws of Wisconsin, 1965) authorizes the DNR, Division of Enforcement, to establish and upgrade minimum standards for floodplain regulations.

Study Area Description

Willow Creek is located in east central Waushara County. Waushara County is located in central Wisconsin. The study area consists of the floodplain adjacent to 13.5 miles of Willow Creek.

The downstream study limit is the east section line of Sec. 7, T. 18 N., R. 13 E. The upstream study limit is approximately 1200 feet north and west of County Highway S bridge located on the section line between sections 1 and 12, T. 18 N., R. 11 E.

The drainage area is as follows:

Upstream Study Limit
Above Cedar Springs Creek
Above Warren Ditch
Downstream Study Limit (Auroraville)

46.15 square miles
52.60 square miles
65.60 square miles
73.50 square miles

Willow Creek is in Hydrologic Unit 04030202-180.

The climate is typically continental. January temperatures average 19 degrees F. July, the warmest month, has an average temperature of 73 degrees F. The average maximum for July is 85 degrees F with the average minimum at 61 degrees F. Precipitation averages 29 inches per year (7).

The soils of the watershed consist mostly of Plainfield-Okee-Richford association, described in the General Soil Map section of the Waushara County Soil Survey, which are strongly sloping to steep, somewhat excessively drained and excessively drained sandy soils on moraines. The lowlands are of the Kingsville-Mechan association, whose soils are nearly level and gently sloping, poorly drained and somewhat poorly drained, sandy, soils on outwash plains.

Natural and Beneficial Floodplain Values

The portion of Willow Creek in the study area is a class II brown trout stream up to the bridge in Section II in Warren Township. The Auroraville millpond has a surface area of 208.6 acres, an average depth of 3.5 feet, and is the third largest body of water in the county. Major fish species include northern pike, largemouth bass, bluegills, perch, sunfish and bullheads. Several waterfowl species and shorebirds use the millpond and surrounding wetlands.

Willow Creek flows through wooded corridors of willow, ash, boxalder and aspen with many adjacent wetlands of alder, cattails, and sedges with scattered open water areas. These wetlands provide important habitat for woodducks, sandhill cranes; mallards, bluewinged teal and other waterfowl, woodcock, muskrats, beaver, racoon, and mink. White-tailed deer, ruffed grouse, cottontail rabbits, fox, otter and numerous songbirds and raptors are also found in the area.

Along with providing wildlife habitat, the undeveloped floodplain and wetlands provides a buffer to help filter runoff from adjacent fields and provides a natural storage area for large amounts of flood water during peak flows.

There are two species in the study area that are listed on the state watch list; the pugnose shiner and the lake chubsucker. Both of these fish species have been found in the Auroraville millpond. They are in jeopardy statewide because of excessive siltation and turbidity. Floodplain management will help ensure the survival of these species. There is no prime farmland in the study area. There are no sites on the National Register of Historic places. Willow Creek is not listed as being eligible for the National Wild and Scenic Rivers Program.

Flooding Problems

No significant flash flooding occurs on the Willow Creek system. The numerous flowages, swamps, depressions, lakes and sandy soils store and release the water over a prolonged period. The increase in flow is handled by removing stoplogs or panels from the dam. Farmland

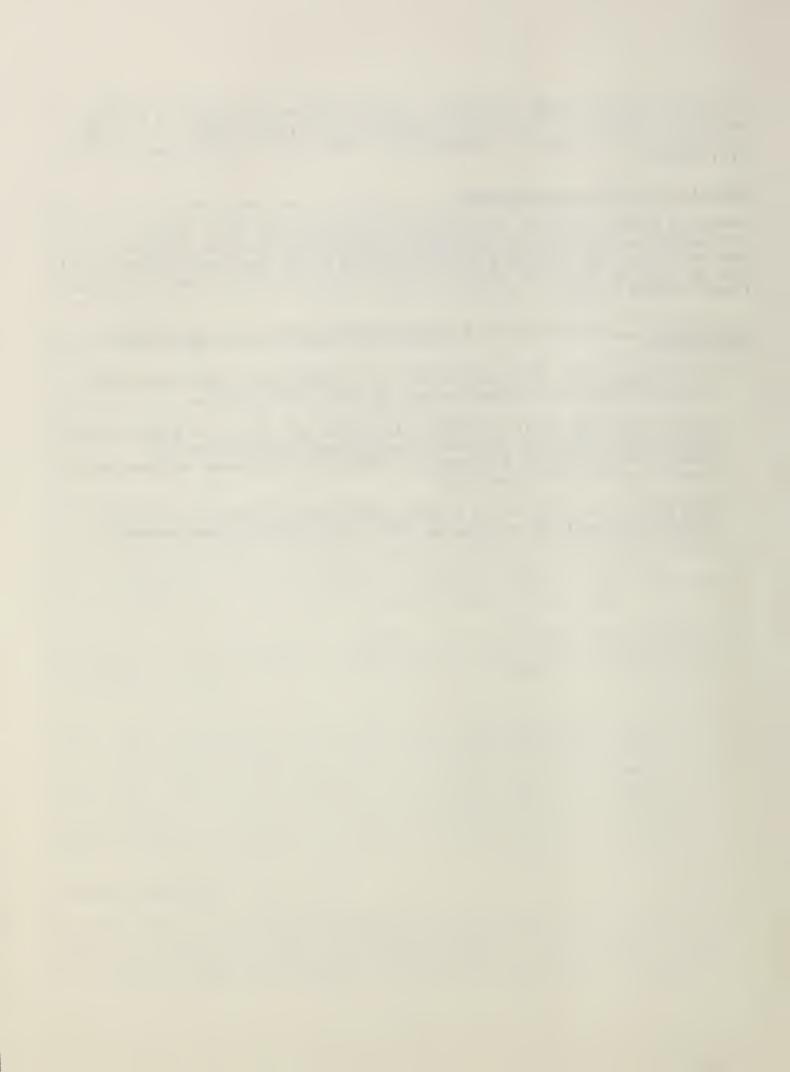
along the Warren Ditch will flood if the dam at Auroraville is not operated properly or the flow exceeds the spillway capacity. A new structure is being built, which should make the operation more efficient.

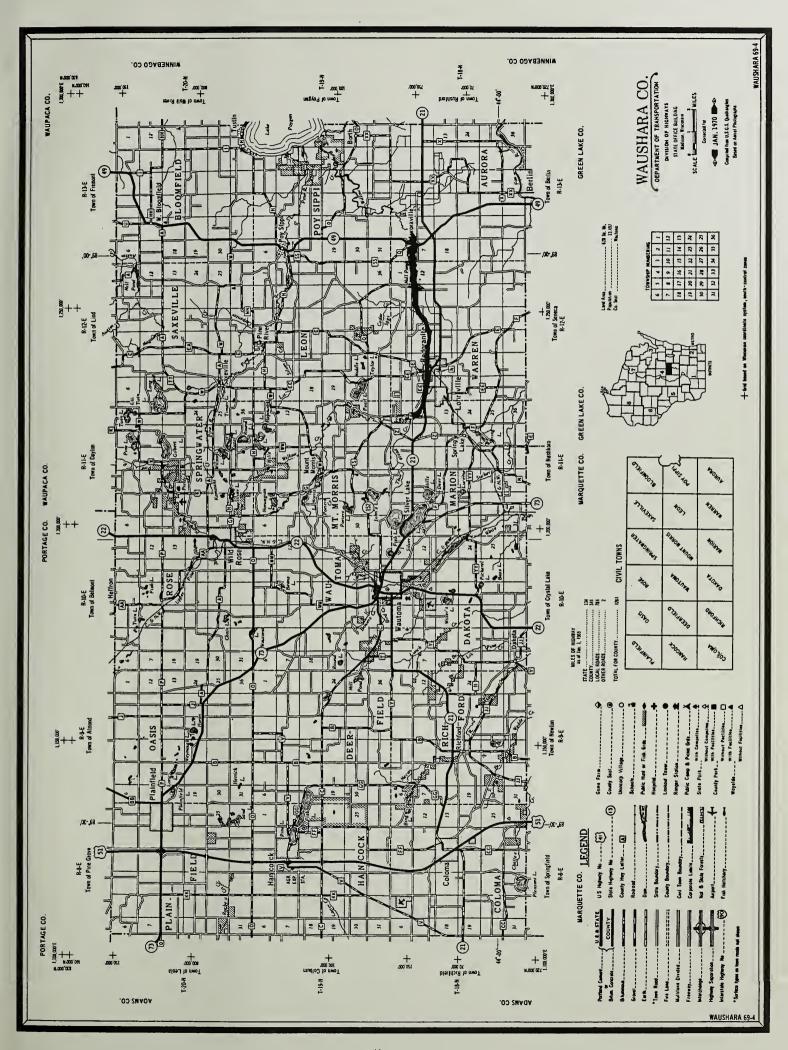
Existing Floodplain Management

Waushara County adapted a floodplain ordinance in 1967. The ordinance was approved by state and federal agencies in May 1986. The county is ineligible for the National Flood Insurance Program and cannot receive disaster assistance or make federally insured loans in flood hazard areas.

Alternatives for Mitigating Flood Damages to Existing and Future Development

- A. The county will incorporate the floodplain maps from this study into a floodplain ordinance and provide enforcement.
- B. The county can apply existing standards set forth in the county's subdivision control ordinance to regulate development in nonsuitable areas and minimize erosion and diffused surface water runoff within the watershed.
- C. The county can establish conservancy districts for those areas highly susceptible to erosion and unsuitable for development.





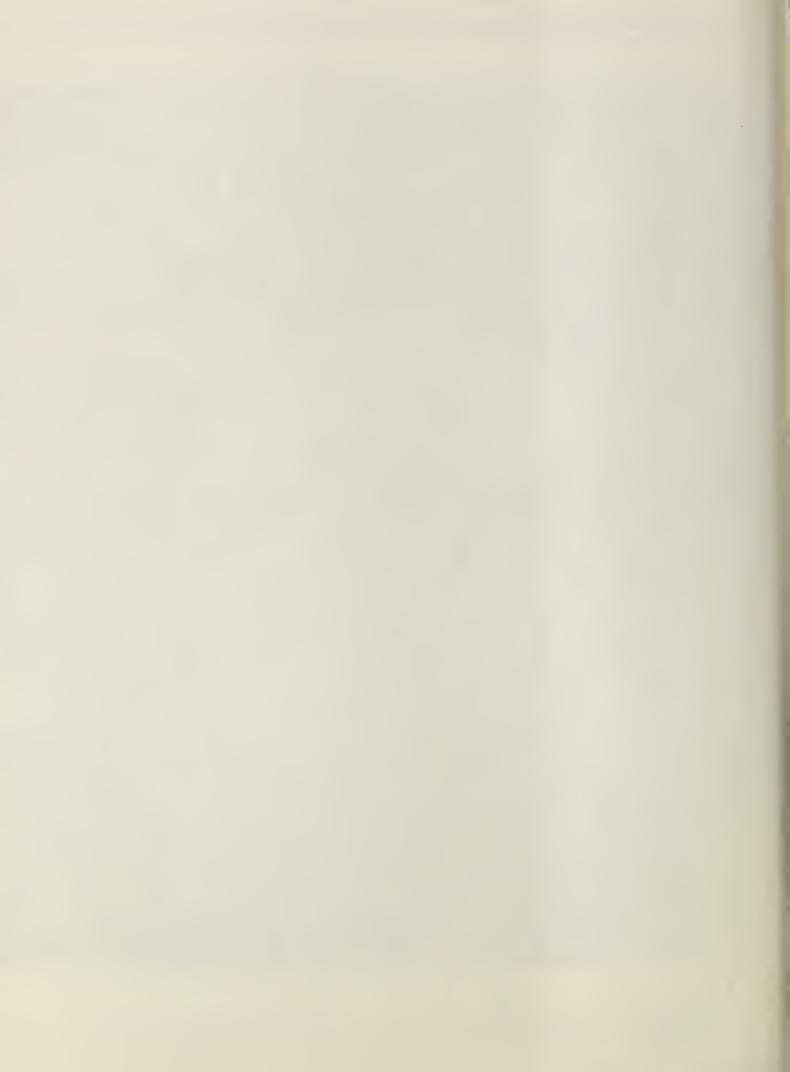


Appendix A

FLOOD BOUNDARY MAPS



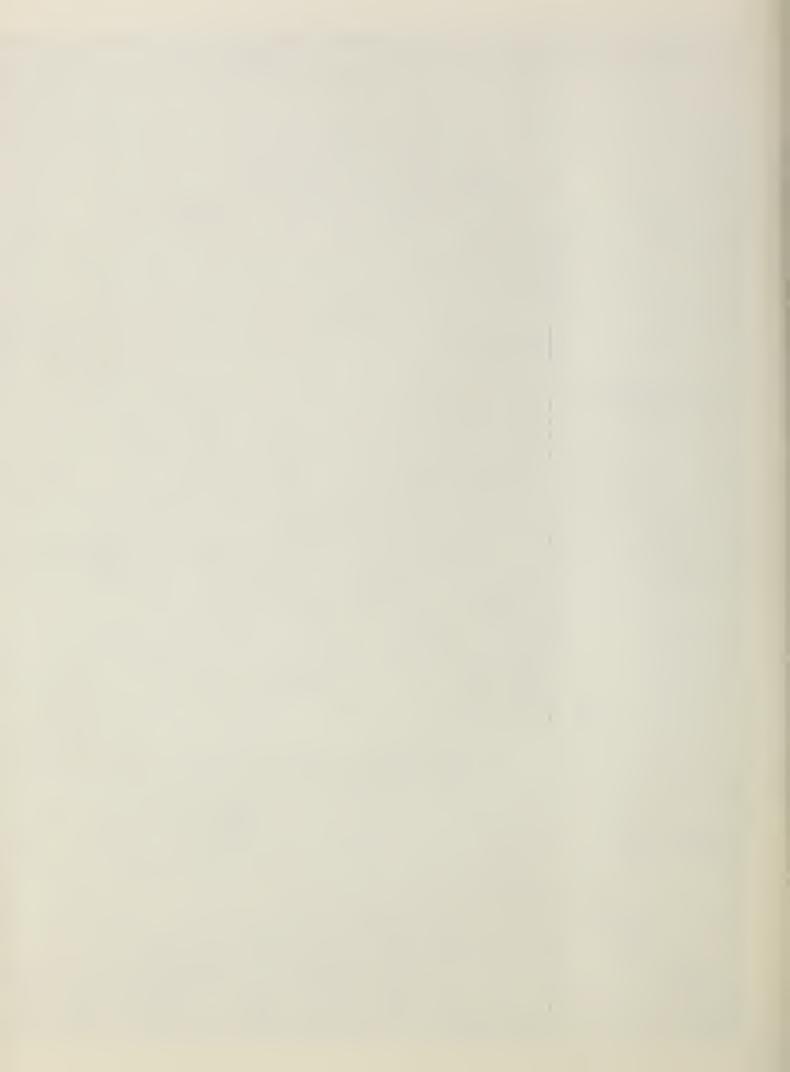




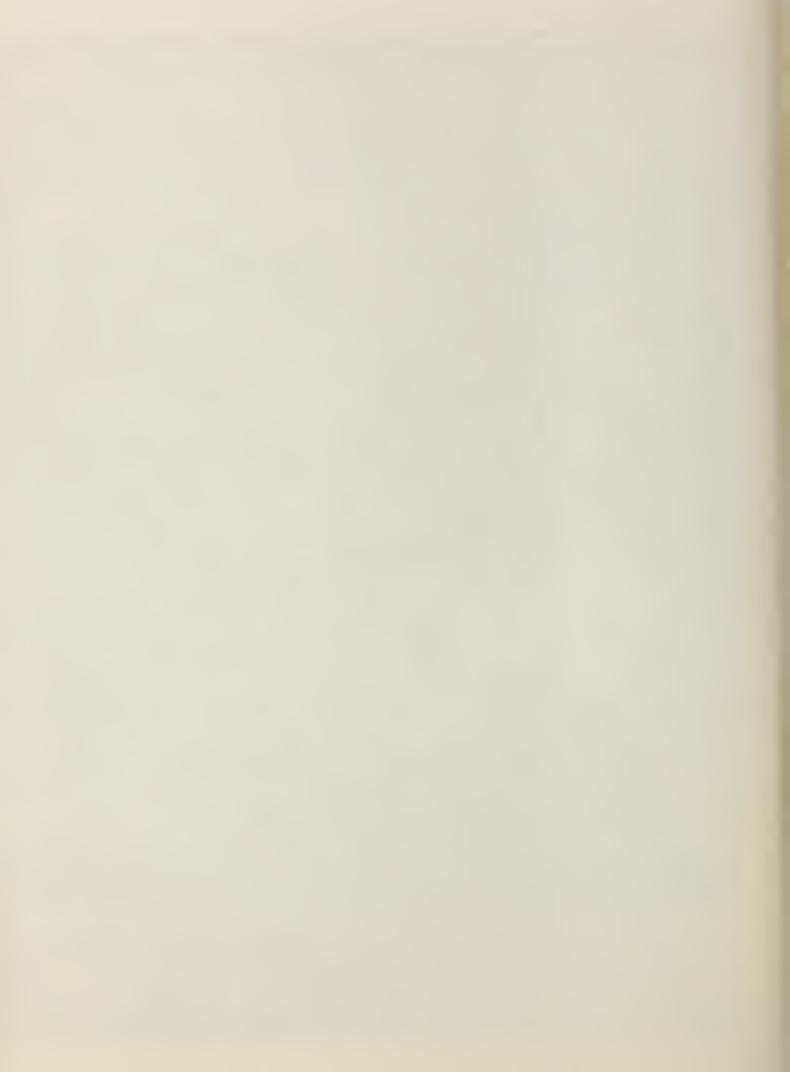




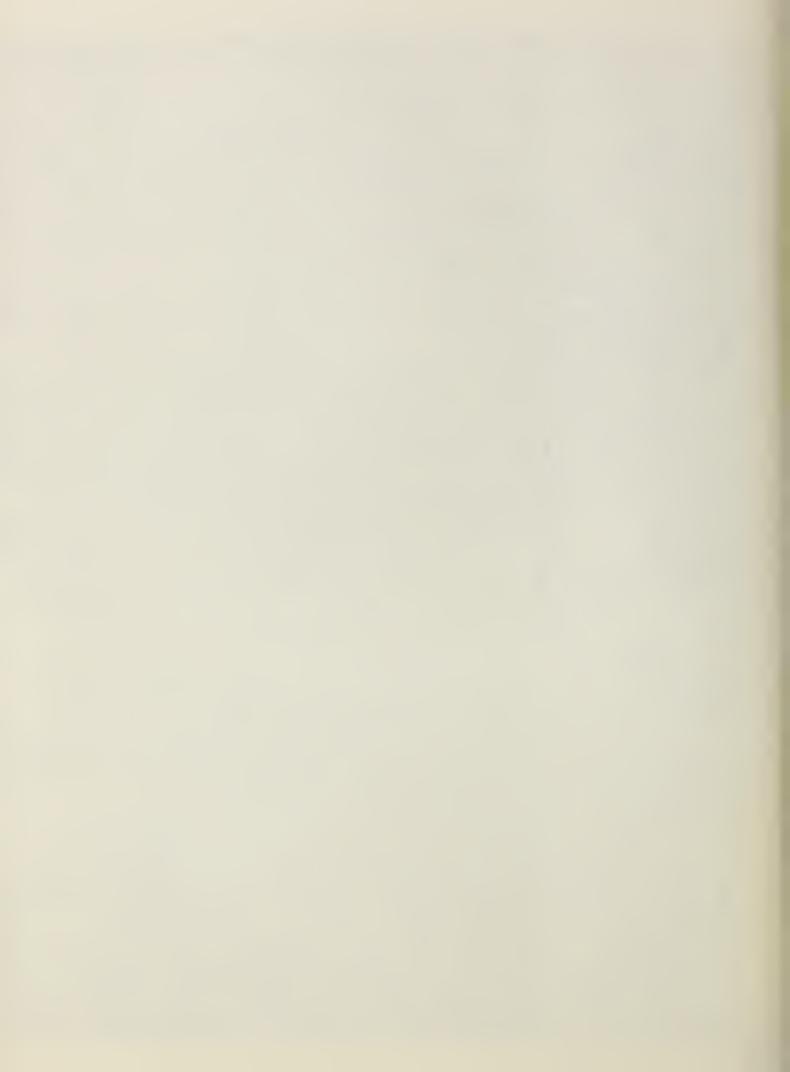








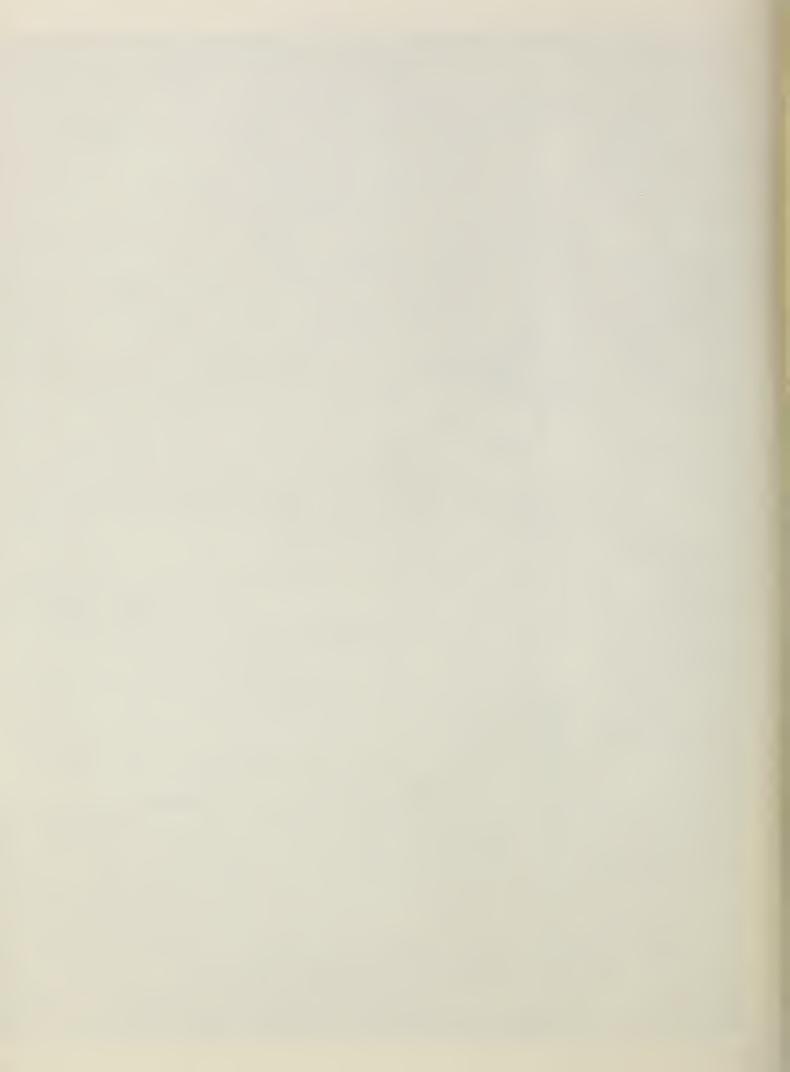




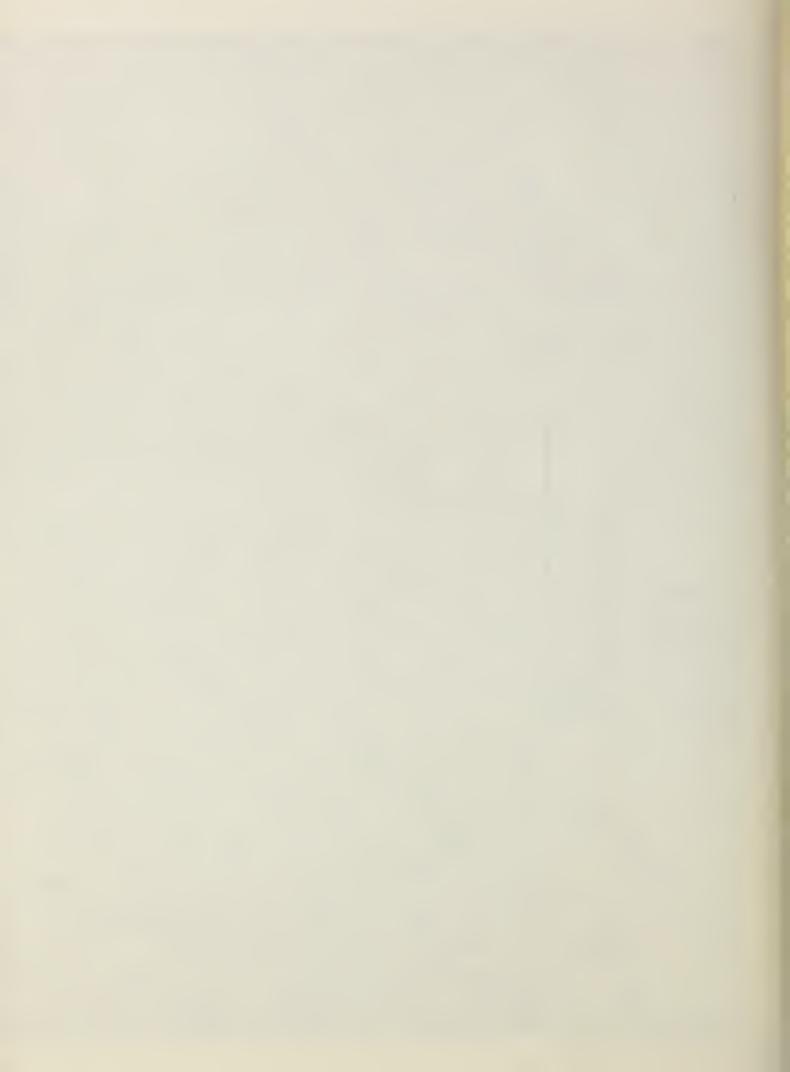




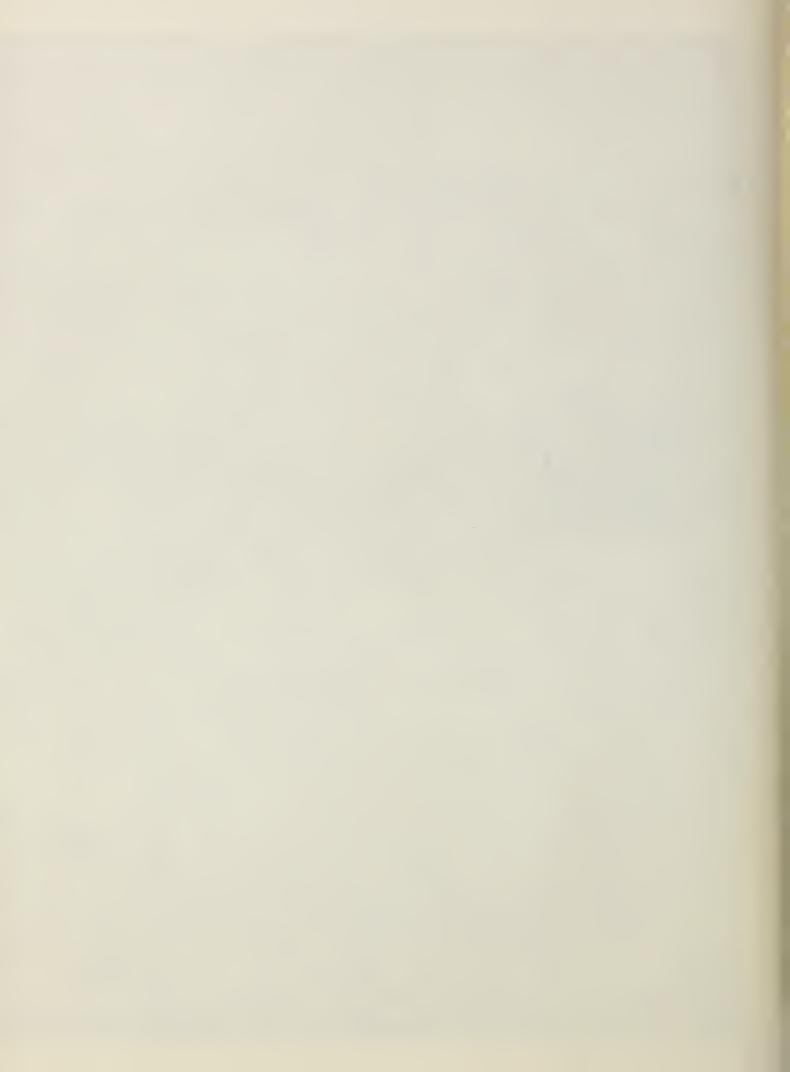




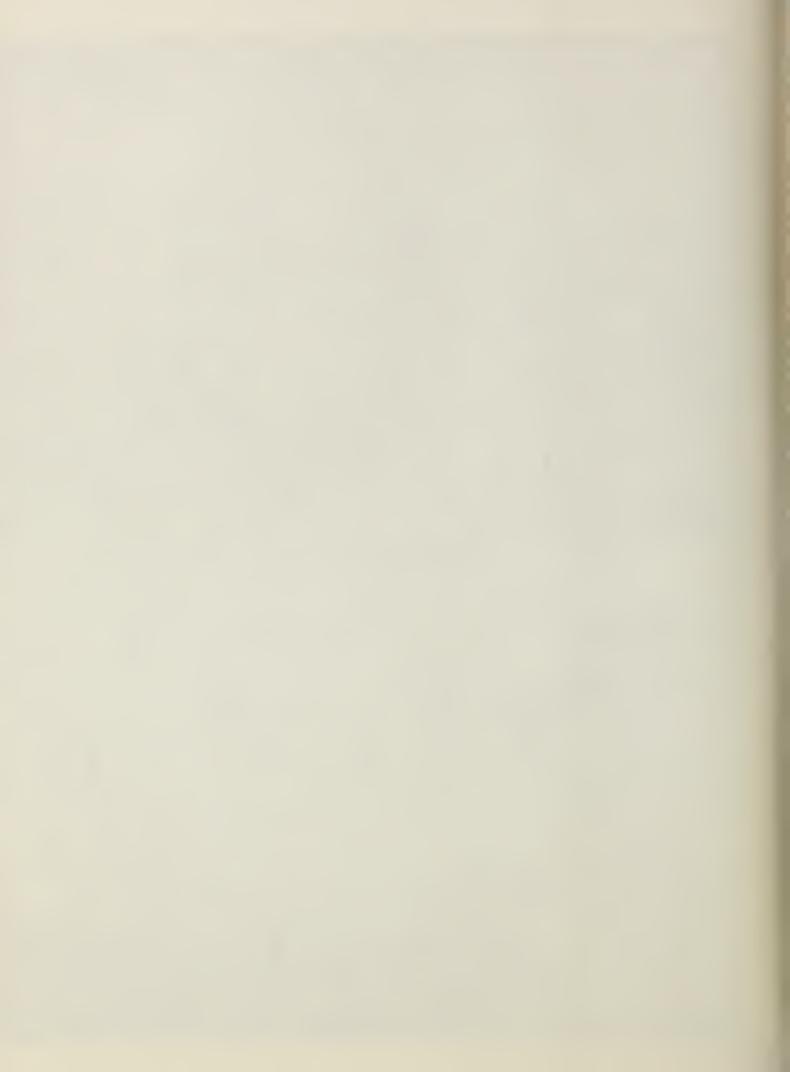




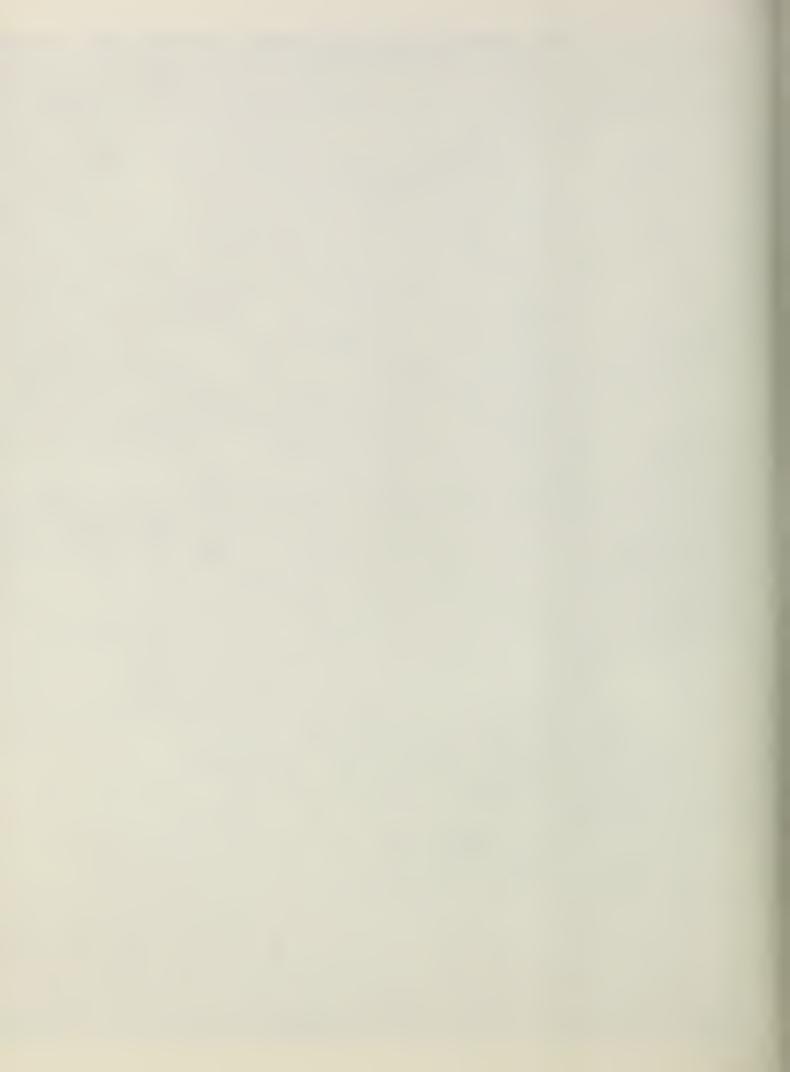








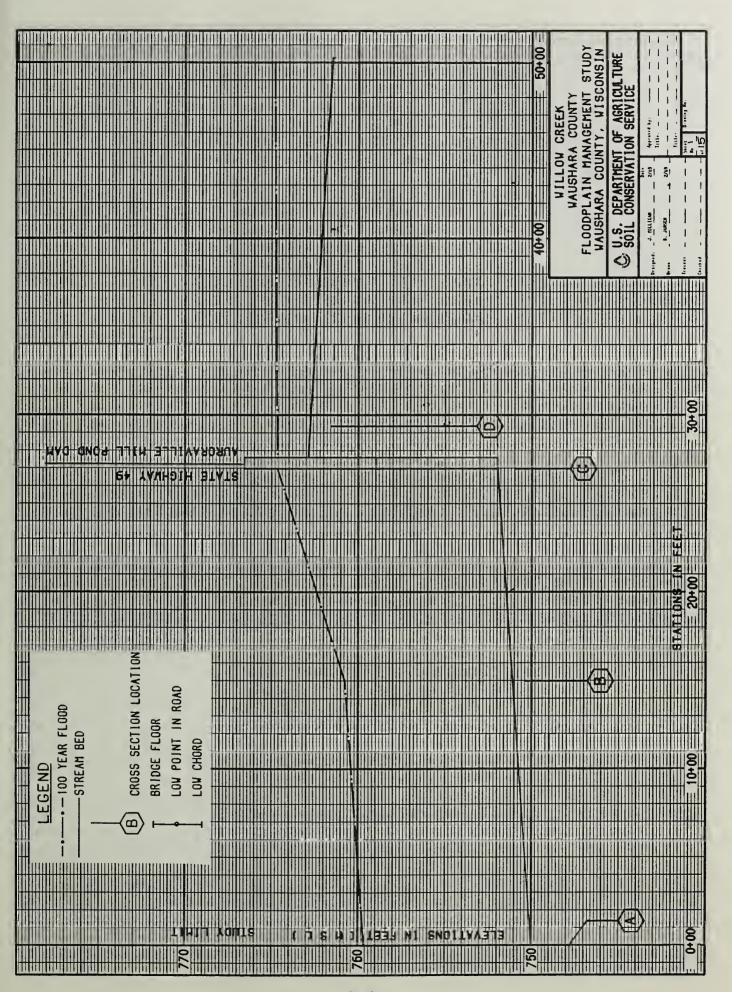


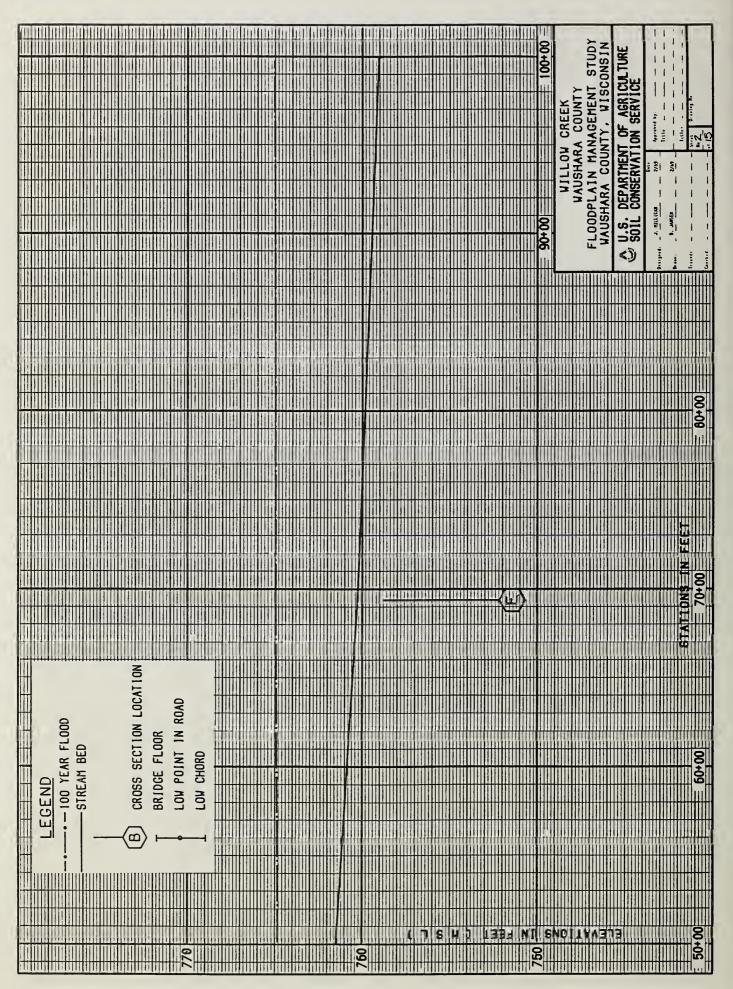


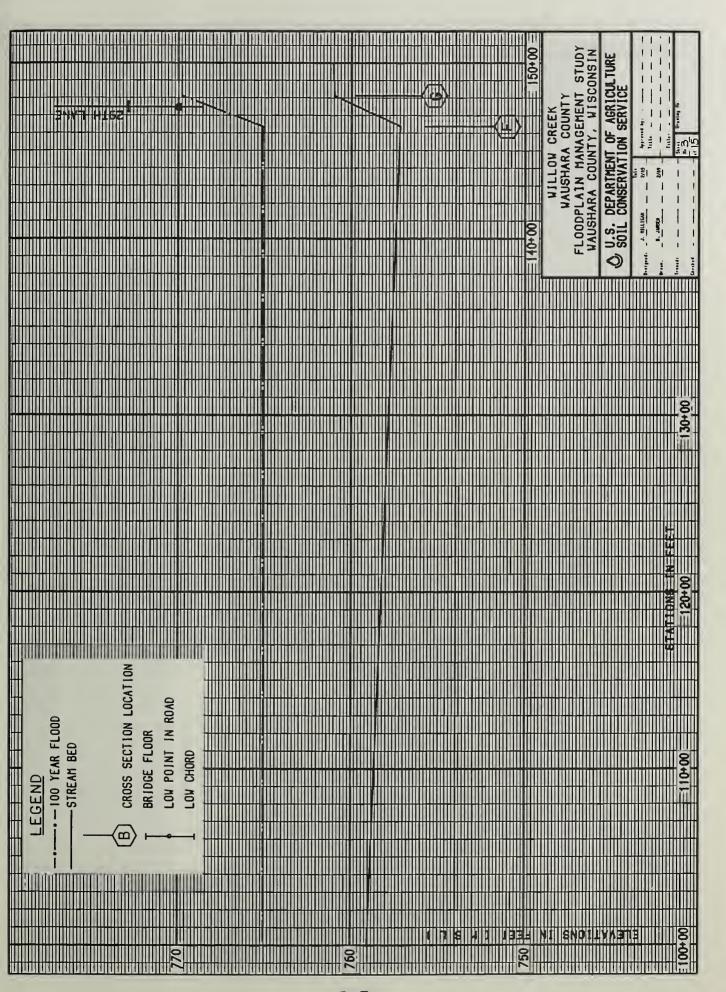
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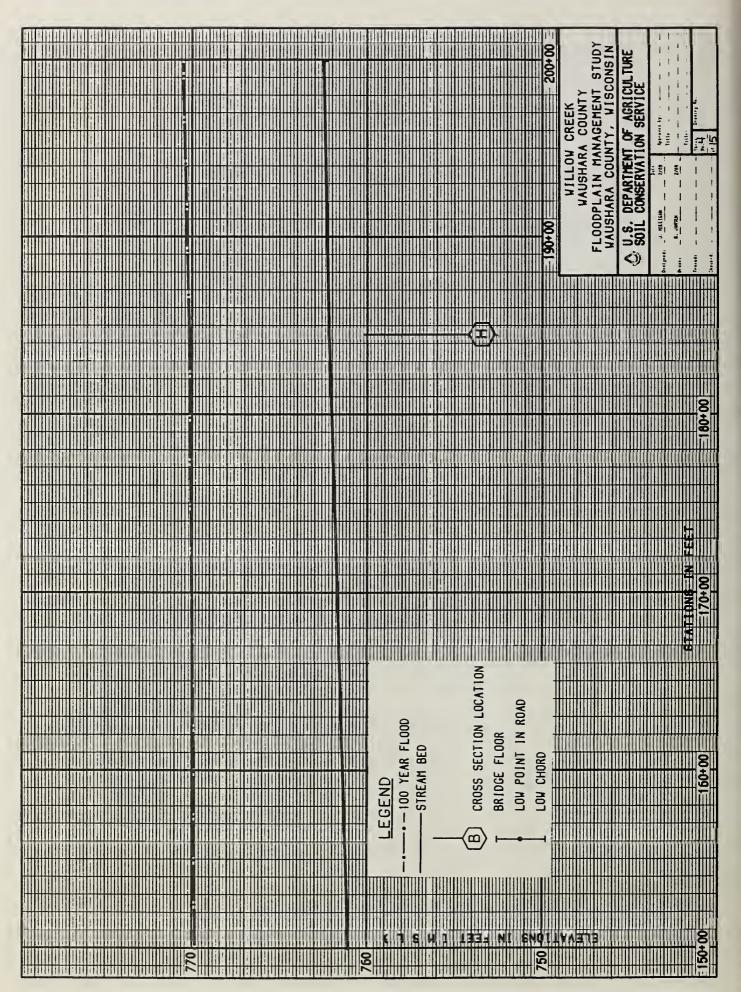
FLOOD PROFILES

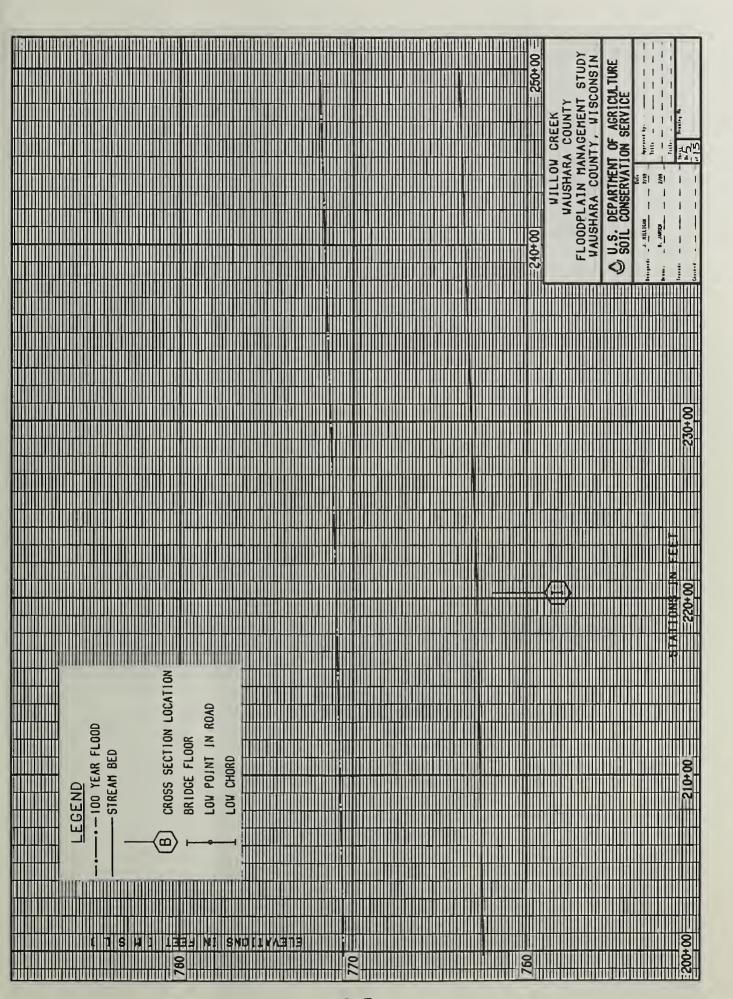


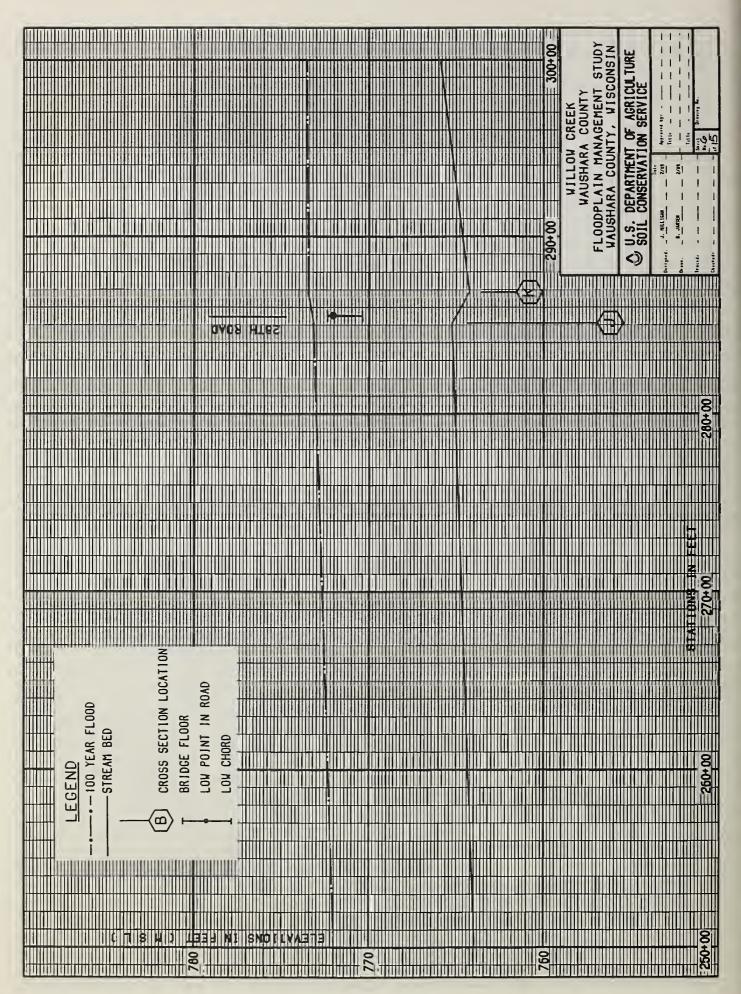


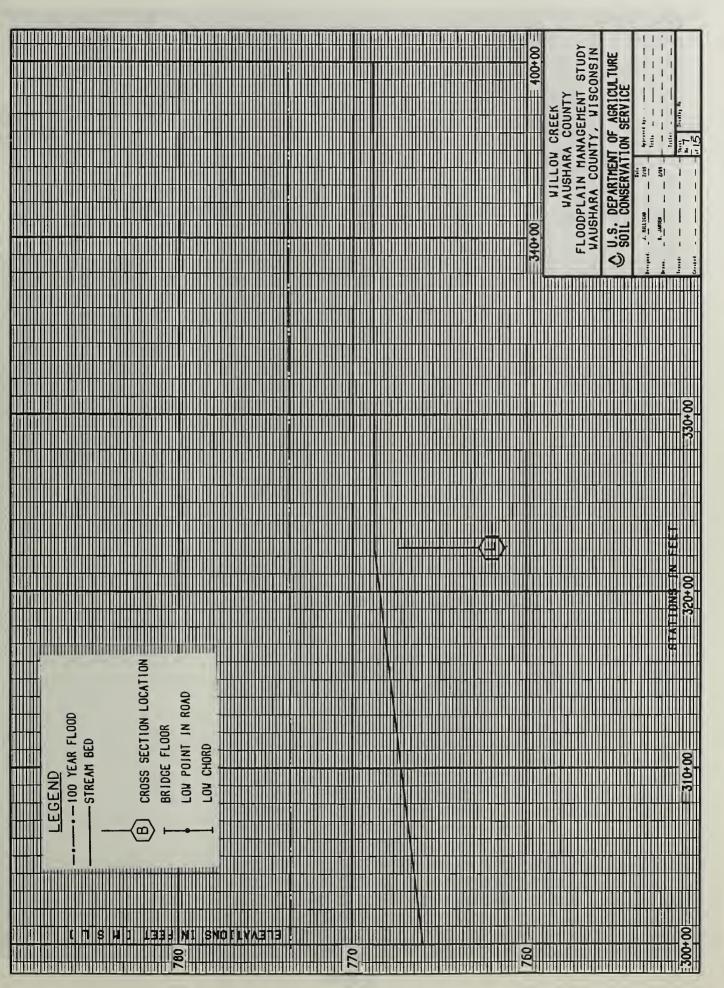


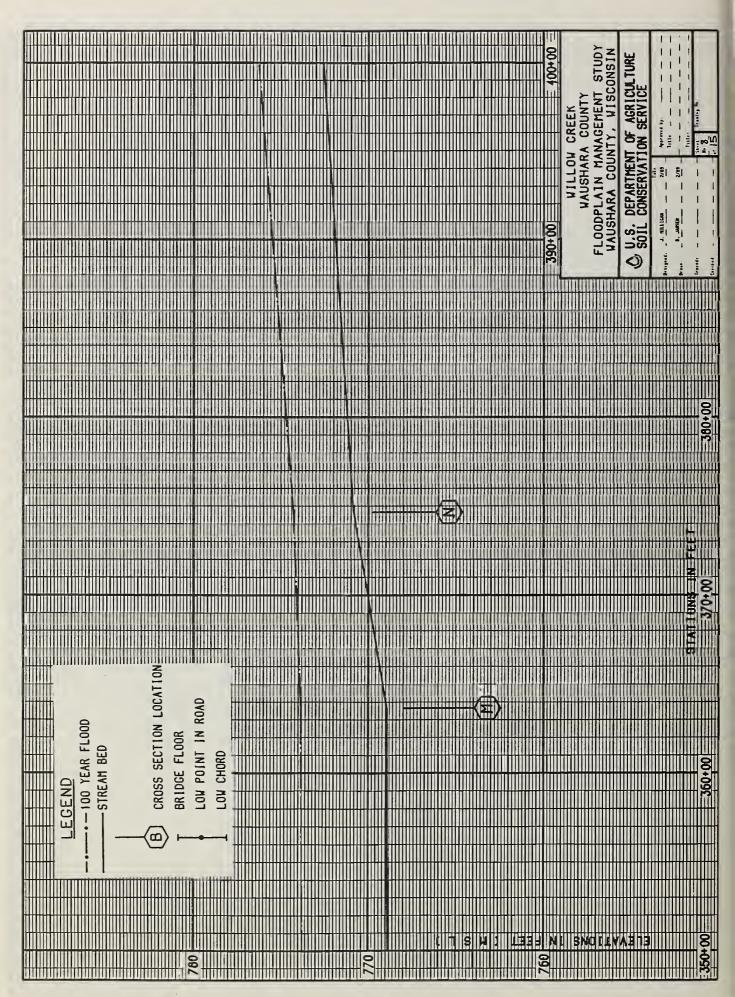


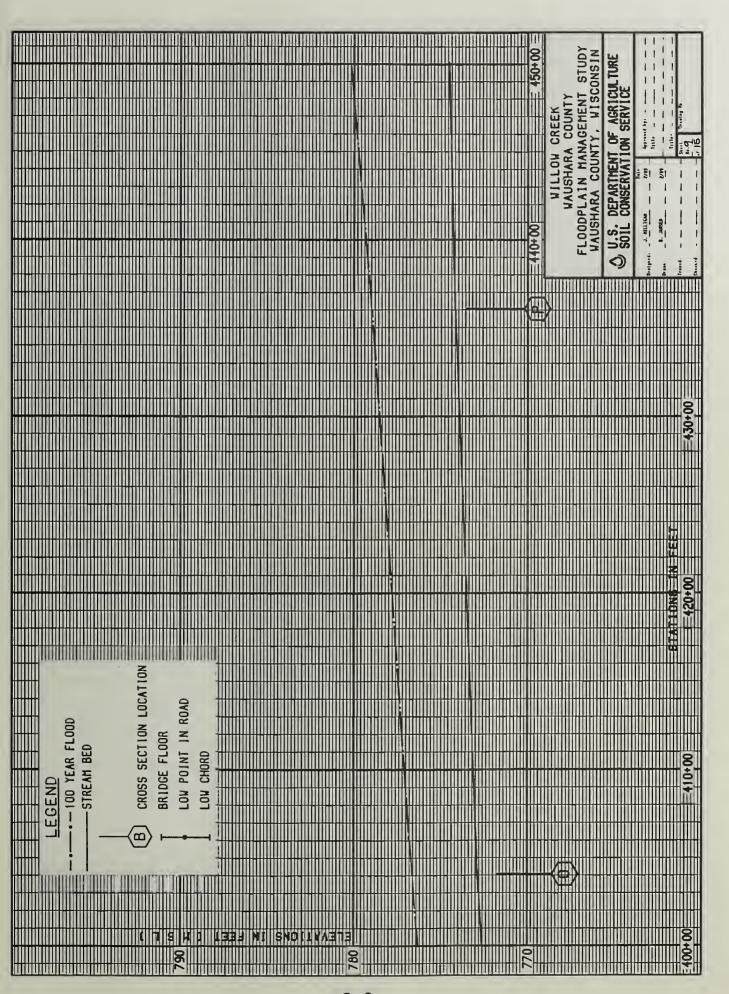


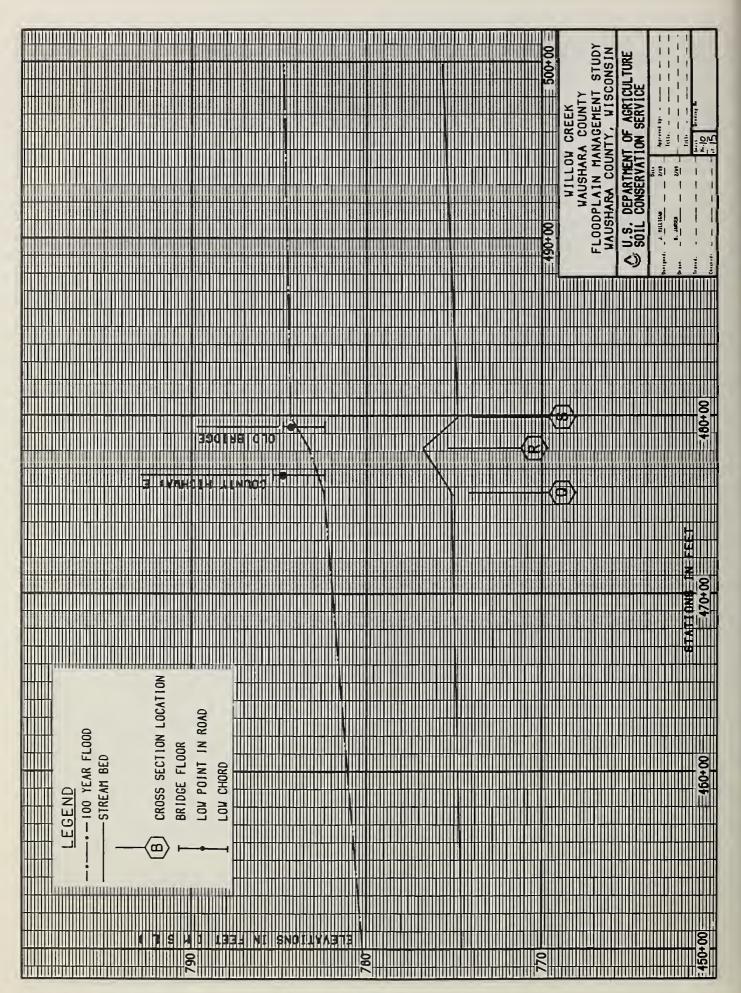


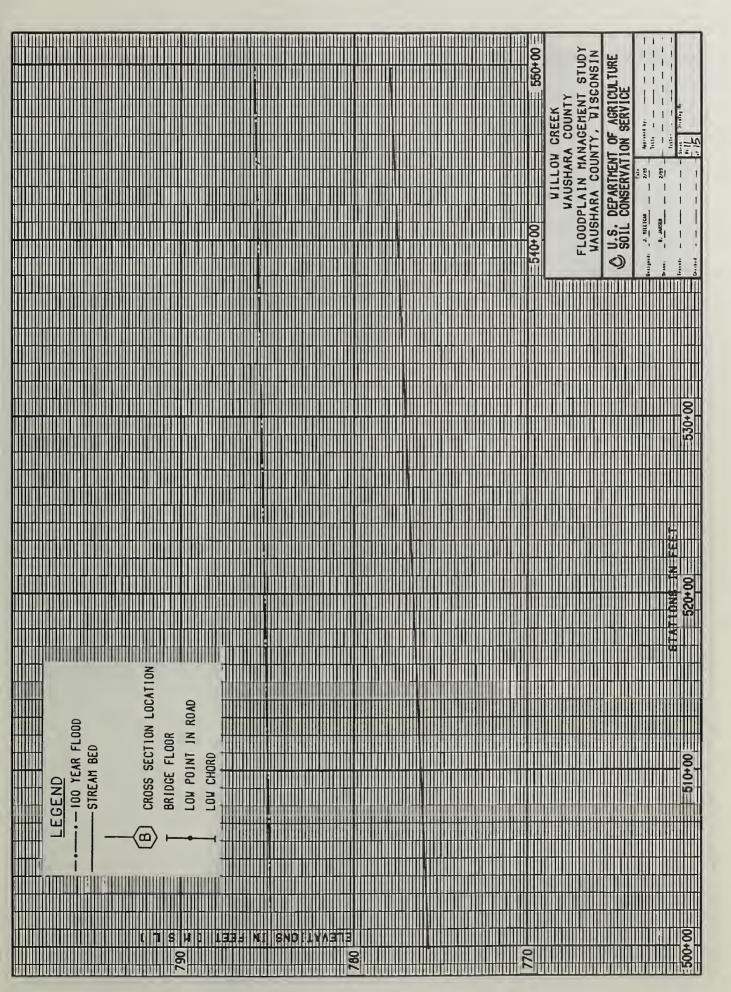


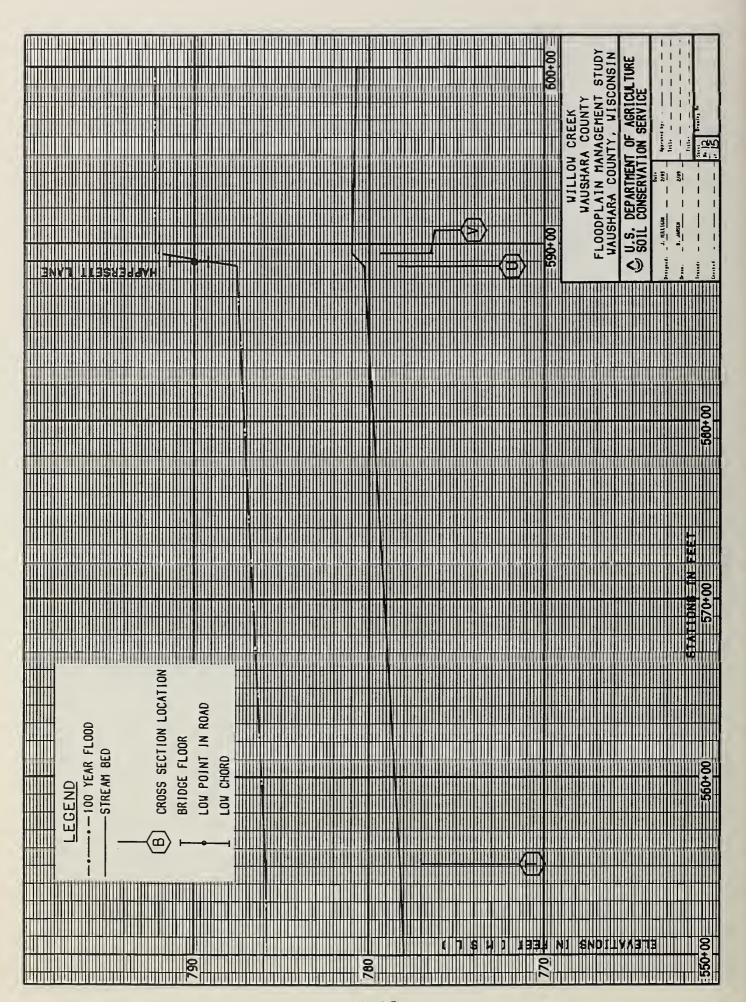


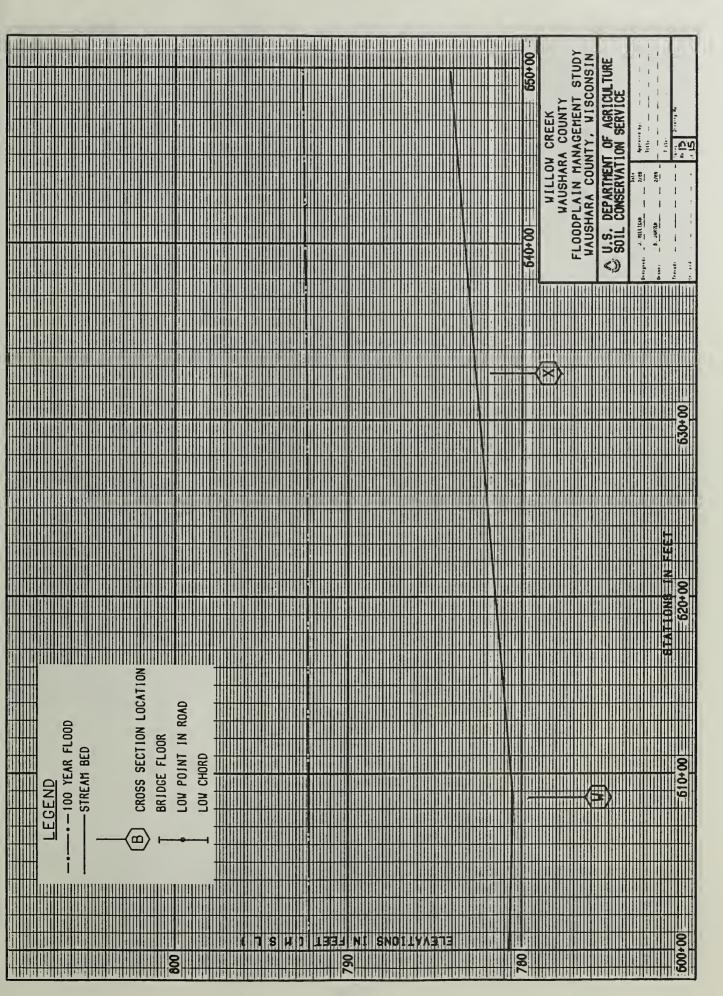


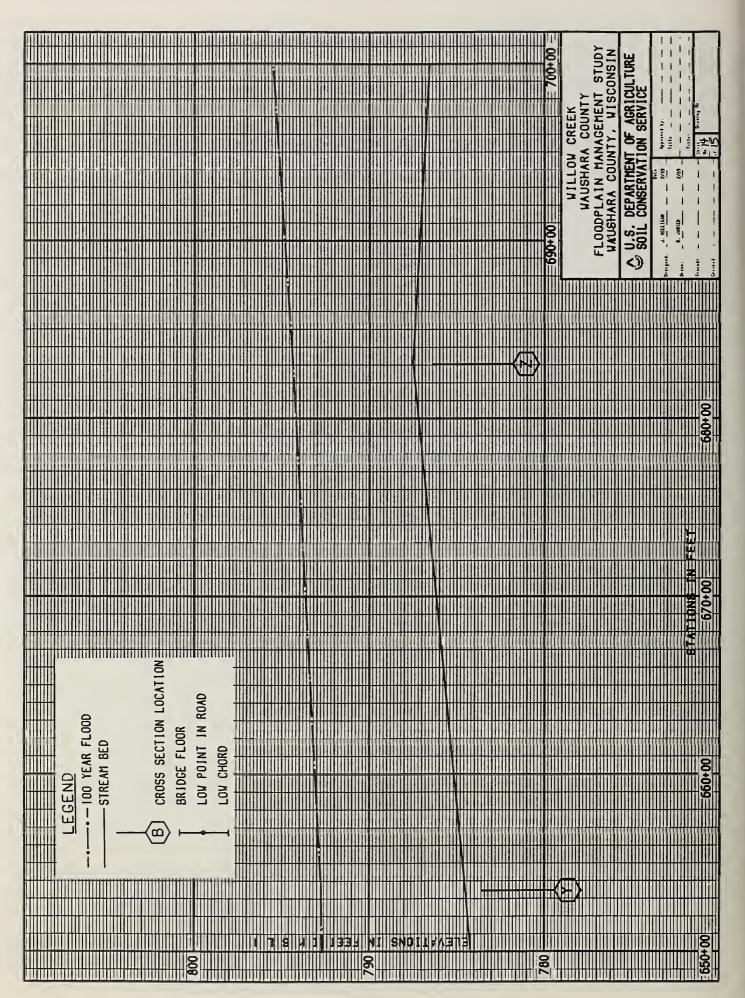


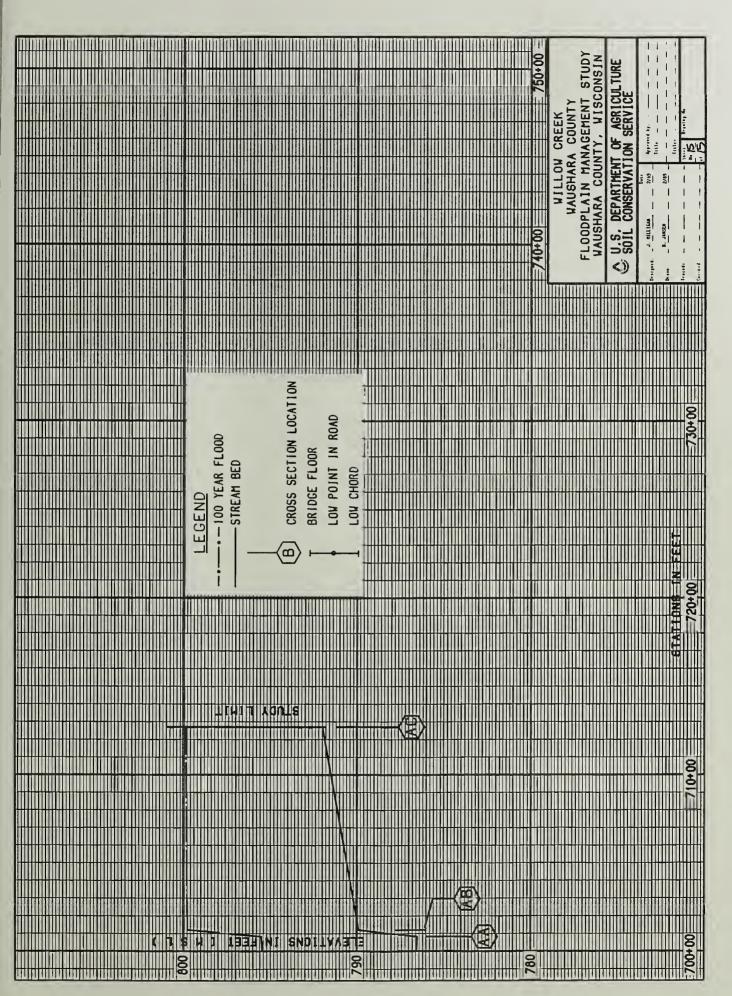










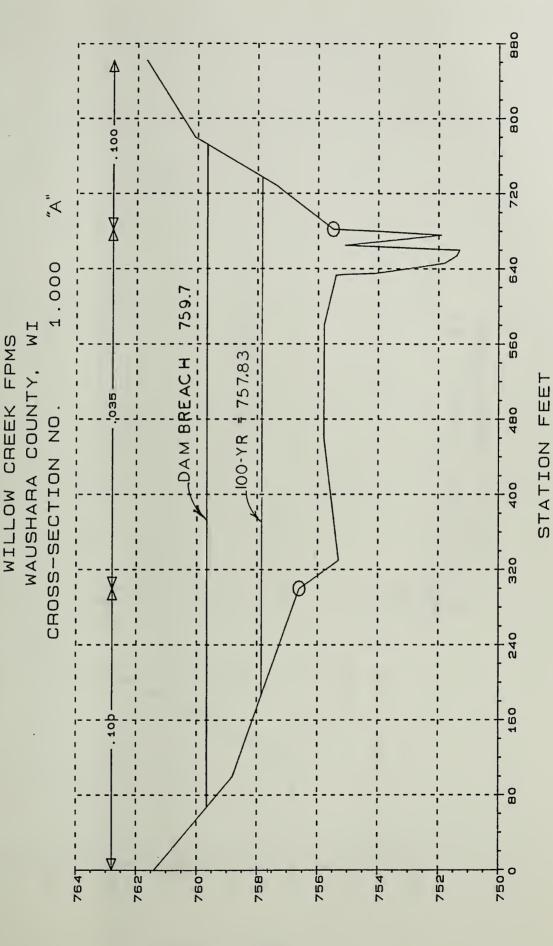




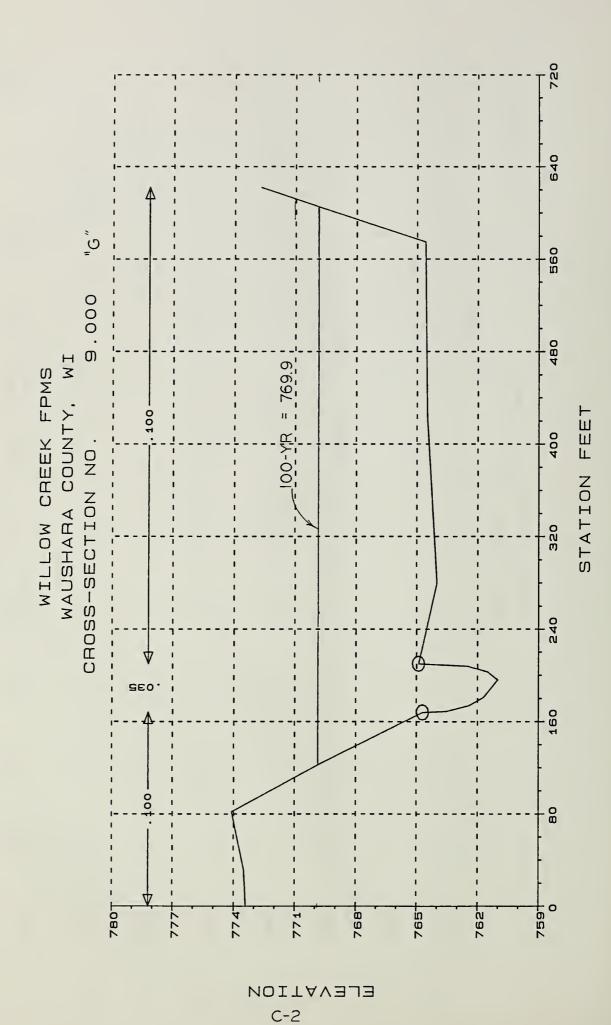
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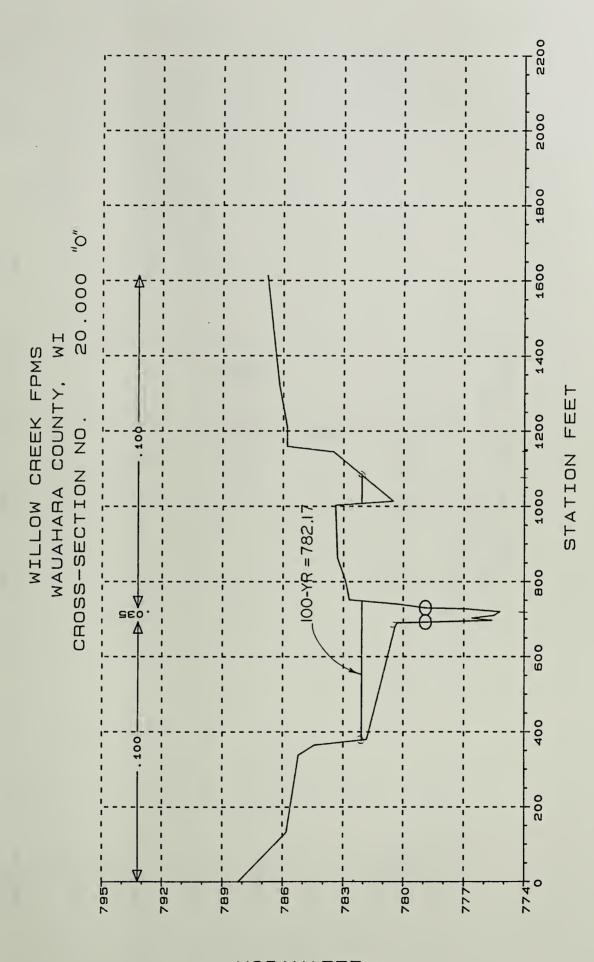
TYPICAL SECTIONS



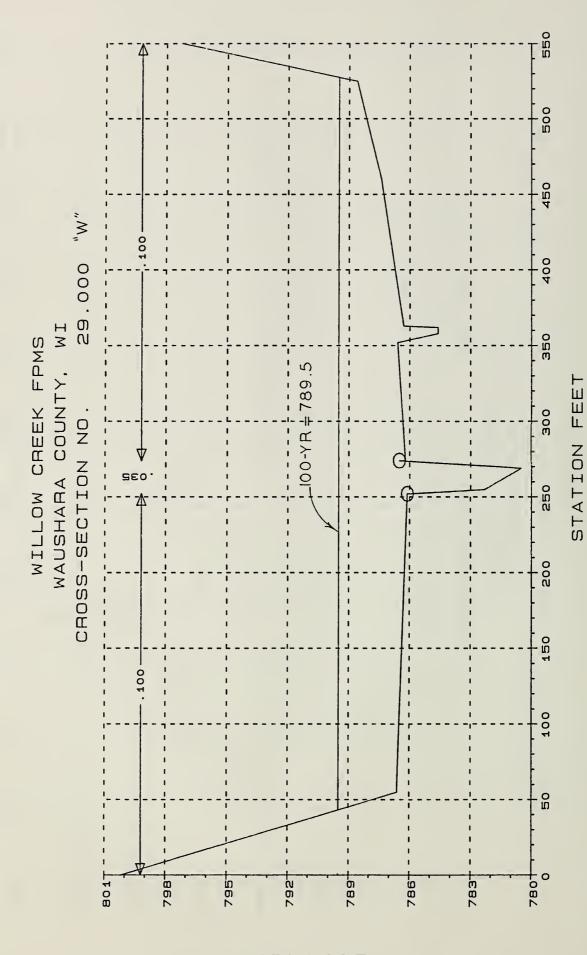


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Appendix D

ELEVATION REFERENCE MARKS



ELEVATION REFERENCE MARKS

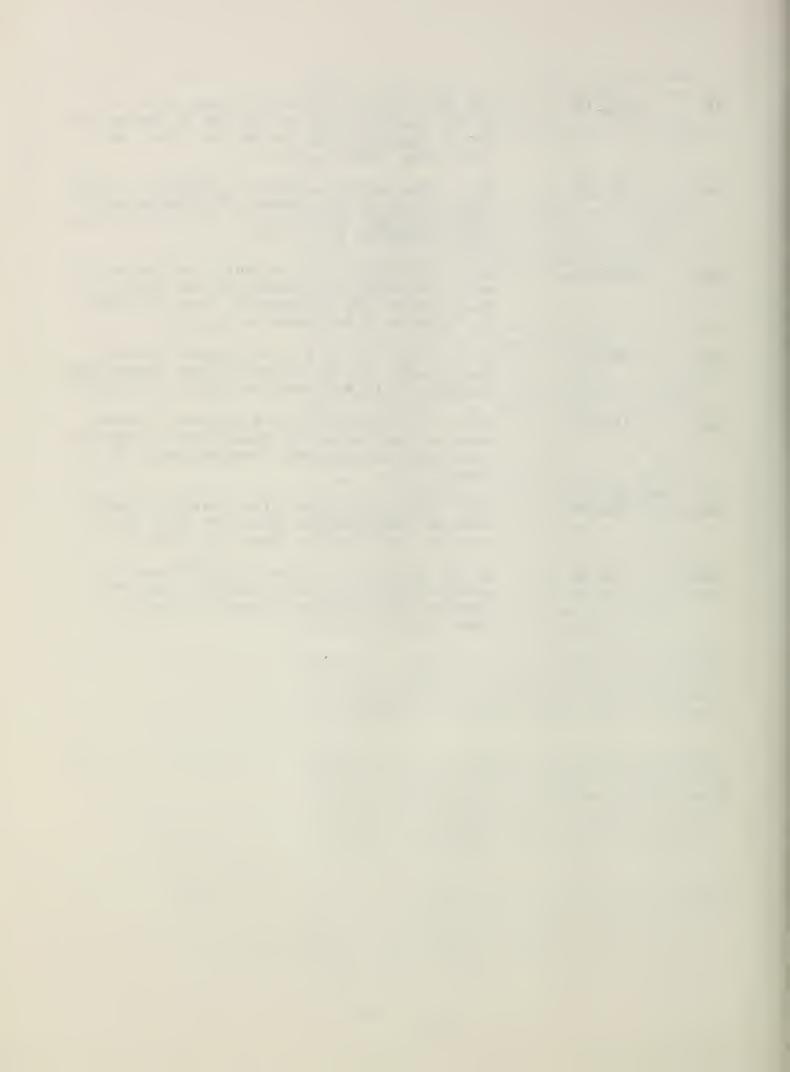
Reference Mark	Elev. (MSL)	Description
1	767.22	USGS BM-J-98 At Auroraville on the east side of Highway 49. A standard brass plate, located on concrete intake to turbine, located in Olsen's Mill.
2	788.535	USGS BM-D-98 Located in Red Granite opposite the southwest end of the Chicago & Northwestern Railroad station at the crossing of State Highway 21, 80 feet southeast of the centerline of the track, 33 feet northeast of the centerline of the highway, and 6 feet northeast of a power transmission line pole located on the north side of Main Street.
3	767.35	TBM-l The top of a l" dia. iron pipe, approximately 18" above ground, 20 feet east of the SE corner of the brown tavern located on the north side of Cottonville Court at the junction with HY-49 in Auroraville. Page 4, Book 1.
4	759.25	TBM-2 The top of the north end of a 24" dia. culvert pipe crossing Cottonville Court Road at a curve to the east, approximately 1000 feet east of Auroraville. Page 4, Book 1.
5	758.21	TBM-3 A spike on the west side of the west pole of two poles of a power transmission line on the north side of Cottonville Court at cross section no. 2(B). Page 4, Book 1.
6	769.69	TBM-4 A spike on the east side of power pole no. 1540 on the north side of Cottonville Court at cross section no. 1(A).
7	766.25	TBM-5 A nail on the south side of South Brey Park sign post on the north side of Cottonville Court, approximately 900' west of Auroraville. Page 5, Book 1.
8	768.25	TBM-6 The top south end of a 21" dia. cmp culvert, approximately 75 feet west of a small white house on the north side of Cottonville Court. Approximately 1200 feet west of Auroraville. Page 6, Book 1.

9	770.94	TBM-7 The top east end of a 12" dia. cmp under a field entrance to the north of Cottonville Court and approximately 250 feet east of a brown house and red barn. Page 6, Book 1.
10	766.84	TBM-8 The east flowline of a 30" dia. cmp culvert, located under the Wessley Belter Driveway entrance. Page 6, Book 1.
11	764.69	TBM-9 The top south end of a 30" dia. cmp culvert crossing Cottonville Court at a curve to the south on 31st Ave. Page 6, Book 1.
12	770.23	TBM-10 A spike on the east side of a power pole, located 50 feet south of 29th Lane bridge on the west side of the road. Page 8, Book 1.
13	770.68	TBM-ll The top south end of "buried" cmp culvert, located approximately 300 feet east of Marshall residence on 29th Court. Page 8, Book 1.
14	775.68	TBM-12 A spike on the north side of power pole no. 18-12-11 38/39, located on the south side of Cottonville Court, approximately 70 feet west of the intersection with 29th Lane to the south and east. Page 8, Book 1.
15	772.92	TBM-13 A nail on the north side of Wisconsin Power & Light Co. (WP&L) power pole number 26/39, approximately 50 feet west of the D. Daniels mail box on the south side of Cottonville Court on the south end of cross section no. 10(H). Page 9, Book 1.
16	772.69	TBM-14 A nail on the north side of Wisconsin Power & Light Co. power pole no. 3/38 on the south side of Cottonville Court, approximately 250 feet east of curve to south. Page 9, Book 1.
17	779.12	TBM-15 A nail on the north side of WP&L power pole no. 37/2 on the south side of Cottonville Court. approximately 250 feet west of the Hackney residence. Page 12, Book 1.
18	772.44	TBM-16 The top northwest wingwall, 4 inches north of the Armco guardrail, of the 28th Road bridge. Page 13, Book 1.

19	775.05	TBM-17 A nail on the west side of WP&L power pole no. 13/32, on the east R-O-W of 28th Road, west & north of the 28th road bridge. Page 13, Book 1.
20	776.96	TBM-18 A nail on the south side of a yellow no trespassing sign on the NW "40" corner of the SW 1/4 of the SW 1/4 of section 9, T18N, R12E, Warren Township. Page 14, Book 1.
21	775.65	TBM-19 A nail on the south side of a 4"x4" leaning oak "no trespassing" sign post approximately 700 feet east of TBM-18. Page 15, Book 1.
22	799.34	TBM-23 The top of the northeast wingwall of the bridge crossing the Willow Creek and located on CTH-"S". Page 18, Book 1.
23	797.21	TBM-24 The top southeast wingwall of a multiplate arch crossing CTH-"S" and located on "Bruce Creek". Page 18, Book 1.
24	801.96	TBM-27 A spike on the north side of a stump, located on the south side of Cottonville Court, approximately 1100 feet west of the intersection with CTH-"S". Page 20, Book 1.
25	801.16	TBM-29 A spike in power pole no. 22/22, located on the south side of Highway 21 in a row of white pine trees, approximately 800 feet east of the intersection of CTH-"S" and Highway 21. Page 21, Book 1.
26	800.75	TBM-30 The top of a brown western most fencepost, located in a fence on the north side of Highway 21 in a wayside rest area, between Red Granite and the intersection of Highway 21 and CTH-"S". Page 21, Book 1.
27	797.67	TBM-31 A spike on the east side of a single power pole on the north R-0-W of Highway 21, located next to an older cream colored two story house about 200 feet west of cross section 29(X). Page 21, Book 1.
28	790.05	TBM-32 The top of a bolt on the east end of a multiplate arch culvert where the Willow Creek crosses Happersett Lane. Page 23, Book 1.

29	799.53	TBM-33 The top of a 3 inch square wood post, approximately 3 feet above ground on the north side of Highway 21 at the intersection of CTH-EE. TBM-33 is located on the south end of cross section 25(U). Page 24, Book 1.
30	790.62	TBM-34 An "X" mark on the concrete side walk, approximately 4 feet south of the west telephone booth located at the city park on the north side of Highway 21, approximately 100 feet west of the intersection with Pine River Street. Page 26, Book 1.
31	764.98	TBM-42 The top of section corner post for sections 6,5,7,8. Page 37, Book 1.
32	790.96	TBM-43 The top bolt of a hydrant located on the NW corner of the intersection of Bonnerman Ave. and Wood Street. Page 40, Book 1.
33	787.75	TBM-44 A spike in the north side of power transmission line pole no. 41/26 located on the south R-O-W of Highway 21 approximately 200 feet SE of the Edgewood Restaurant. Page 40, Book 1.
34	786.27	TBM-45 The top of telephone inspection box, located on the south side of a power pole that is located on the north R-O-W of Highway 21 at the south end of cross section 17 (N). Page 41, Book 1.
35	777.72	TBM-46 A spike in the south side of power pole no. 33/39, located on the north R-0-W of Highway 21, approximately at the south end of cross section 16(M). Page 41, Book 1.
36	773.03	TBM-47 A spike in the south side of WP&L power pole no. 51/39, located in the north R-O-W of Highway 21, opposite the driveway to an auto salvage yard, located in the northeast corner of the SE 1/4 of the NE 1/4 of section 16, T18N, R12E. Page 42, Book 1.
37	778.04	TBM-48 A spike in the south side of power pole no. 13/39, located on the NE corner of the intersection of 28th Lane and Highway 21. Page 8, Book 1.

38	768.18	DNR BM Top of a steel cylinder type bridge support pier under the SW corner of the bridge crossing the Willow Creek on 29th Lane. Page 8, Book 1.
39	792.58	BM-6 The top of a hydrant, located on the north side of Main Street, approximately 600 feet east of Highway 21, in Red Granite Established in Nov. 1975
40	792.33	BM-11 The top of a hydrant, located on the north side of Main Street at the corner of Lang Street and Highway 21. Page 28, Book 1. Village of Red Granite.
41	794.30	BM-16 The top of a fire hydrant located north east of the corner of Pine Street and Highway 21 in Red Granite. Page 26, Book 1.
42	789.27	BM-25 The top of a fire hydrant located west of the corner of Pine River Street and Lafayette Street, Red Granite. Page 27, Book 1.
43	786.56	BM-27 The top of the fire hydrant located west of the sewerage plant on Pine River Street, Red Granite. Page 27, Book 1.
44	790.28	BM-46 The top of fire hydrant located northwest of the corner of Wood and Main Streets, Red Granite. Pages 14 and 28, Book 1.



Appendix E

TABULATION OF
WATER SURFACE ELEVATIONS
DISCHARGES
AND
FLOODWAY TABLES



	500 YEAR	Q ; ELEV CFS ; MSL	1930.00 759.18 1930.00 759.56 1930.00 759.71	2000.00 765.80 2000.00 766.00 2000.00 766.10	2000.00 771.23 2000.00 771.54 2000.00 772.18 2000.00 773.92	570.00 1774.05 570.00 1774.21 570.00 1774.41 570.00 1774.68		1500.00 784.80 1500.00 786.46 1500.00 788.11	1500.00 794.56 1420.00 794.62 1420.00 794.68 1420.00 795.69 1420.00 795.69	1420.00 800.79
	٠- ٧	ELEV ; CF	757.83 1930 758.57 1930 758.77 1930	764.70 2000 764.99 2000 765.14 2000	769.90 2000 770.29 2000 771.16 2000 773.34 2000	.56		72	792.20 1500. 792.30 1420. 792.42 1420. 792.74 1420. 794.37 1420.	99.34
- ELEVATION	100 YEAR	CFS	1550.00 1550.00 1550.00	1550.00 1550.00 1550.00	1550.00 1550.00 1550.00 1550.00	1200.00 1200.00 1200.00	1150.00	1150.00 1150.00 1150.00	1150.00	1100.00 7
DISCHARGE	YEAR	ELEV MSL	757.67 758.37 758.54	764.20 764.55 764.73	769.10 769.55 770.61 772.99		778.79 778.79 782.21		791.30 791.42 791.56 791.97 793.89	798.43 798.60 t.
	20	G S	1360.00 1360.00	1300.00 1300.00 1300.00	1300.00 1300.00 1300.00	1020.00 1020.00 1020.00	970.00		970.00 920.00 920.00 920.00	920.00 920.00 study limi
	YEAR	ELEV MSL	757.15 757.71 757.84) u	767.81 768.43 769.82 772.38	772.70 772.86 773.17 774.07	778.49 778.70 E 781.70 1 782.06	782.40 784.87 786.75 TT LANE	789.31 789.53 789.80 790.61 793.10 793.10 793.10	6
	10	CFS	870.00 870.00		930.00 930.00 930.00 930.00	88888	730.00 700.00 CTY HWY	700.00 700.00 700.00	700.00 600.00 600.00 600.00 600.00	600.00 600.00
SOURCE	DISTANCE 1/		1500	2000 2004 60040 14640	14402 14815 18455 22035 28515	28690 32250 36370 37470 40410	43610 47570 47682 47820 47932	55515 58875 58912	58949 60869 63269 65349 70049	- '
FLOODING SO	CROSS SECTION		∢ ⊞∪	ОШГ	σIμ¬	⊼ ¬ΣΣC	o	ω⊢⊃ .	> ~ × × × &	AC

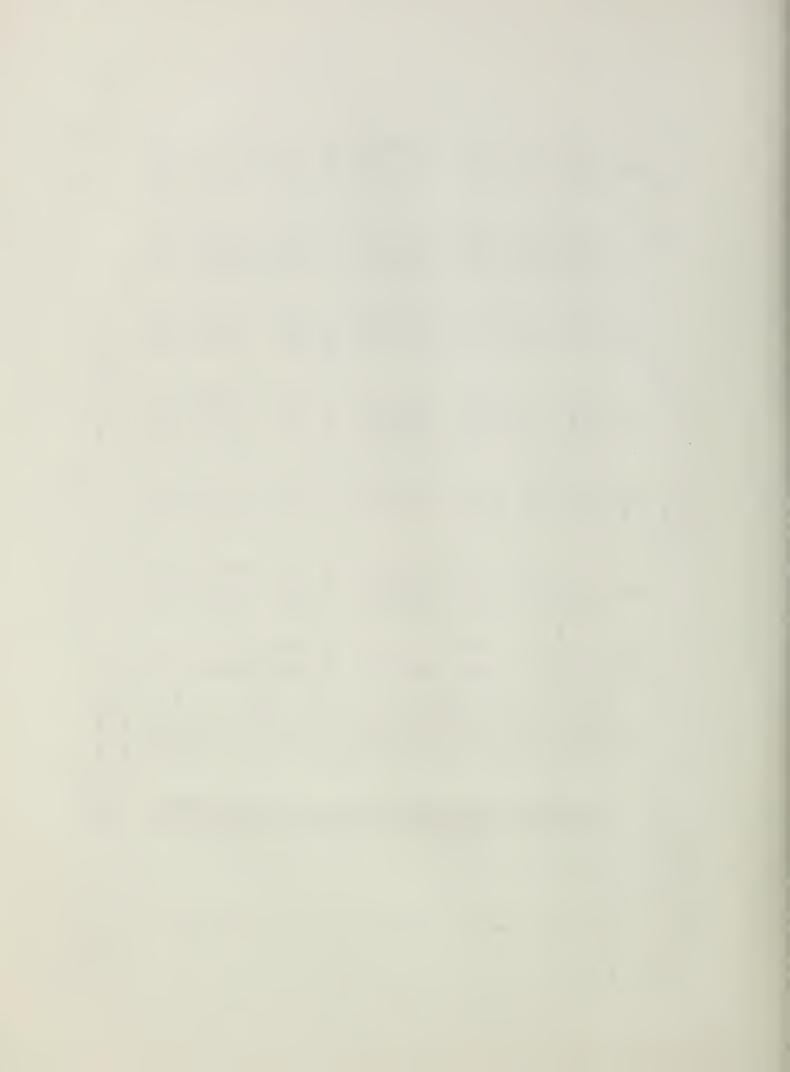


TABLE 2 FLOODWAY DATA

FLOODING SOURCE			FLOODWAY		BASE FLOOD WATER SURFACE ELEVATION 2/			
CROSS SECTION	DISTANCE 1/	WIDTH (FEET)	: SECTION : AREA : (SQUARE : FEET)	MEAN VELOCITY (FEET PER SECOND)	: WITHOUT : FLOODWAY : WITH : DAM BREAK	WITHOUT FLOODWAY	: WITH 3/ : FLOODWAY : AND : DAM BREACH	INCREAS
WILLOW CREEK			· ;	- 		; :	;	
A A	0	549	1096	3.11	759.66	: 757.83	757.83	. 0
n D	1500	560	1 1588	1.86	1 760.77	1 758.57	1 758.57	. 0
L D	2700	298	1372	1.39	764.68	1 758.77	1 758.77	. 0
D.	1 2940 1	340	628	1 2.54	764.70	764.70	1 764.70	. 0
F	1 6940 1	2006	÷ 5557	.40	764.99	1 764.70	764.70	. 0
E E	1 14640	428	1 636	4.31	765.14	765.14	765.14	. 0
e '	14815	482	2494	1.59	769.80	769.80	769.80	. 0
u	1 18455	507	2108	2.02	770.17	770.17	770.17	. 0
ï	22035	704	1 1355	3.20	771.02	771.02	771.02	: 0
i	28515	785	1877	2.59	773.22	773.22	773.22	: 0
ĸ	28690	1392	3454	1 1.40	773.52	773.52	773.52	. 0
ì	1 32250 1	2450	6343	1 .65	773.68	1 773.68	773.68	. 0
M	1 36370 1	2425	1 3358	1.43	773.00	773.00 773.91	773.00	: 0
rı N	1 37470 1	2750	1 1944	2.69	774.34	1 774.34	773.31	. 0
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D	1 43610 1	893	1100	1 4.09	1 778.98	1 778.98	778.98	, U
r 0	1 47570 1	374	717	1 3.76	1 782.46	1 782.46	1 782.46	. 0
ų D	1 47820 1	401		1 2.63	1 783.32	1 783.32	1 783.32	. 0
K C	1 47020 1	494	964		1 784.37	1 784.37	1 784.37	. 0
J	1 55515 1	549	1490	2.08 2.37	1 785.88	1 785.88	1 785.88	. 0
i	1 58875 1	213	1430	1 4.83	1 787.58	1 787.58	1 787.58	. 0
V	1 58949 1	370	1 1948	1.67	792.20	1 792.20	792.20	. 0
M A	1 60869 1	504	1 2822	1.19	792.30	1 792.20	792.30	. 0
7 V	1 63269 1	412	1880	1.13	792.42	1 792.42	792.42	. 0
Ŷ	1 65349 1	199	1 766	3.39	792.74	1 792.74	1 792.74	. 0
7	1 68309 1	307	1 848	3.33	1 794.37	1 794.37	1 794.37	. 0
44			1 639			1 795.57		Ĭ
AA AB	; 70086 ; 70124 ;	204 495		1.49	1 795.57 1 799.78	1 799.78	795.57 799.78	0
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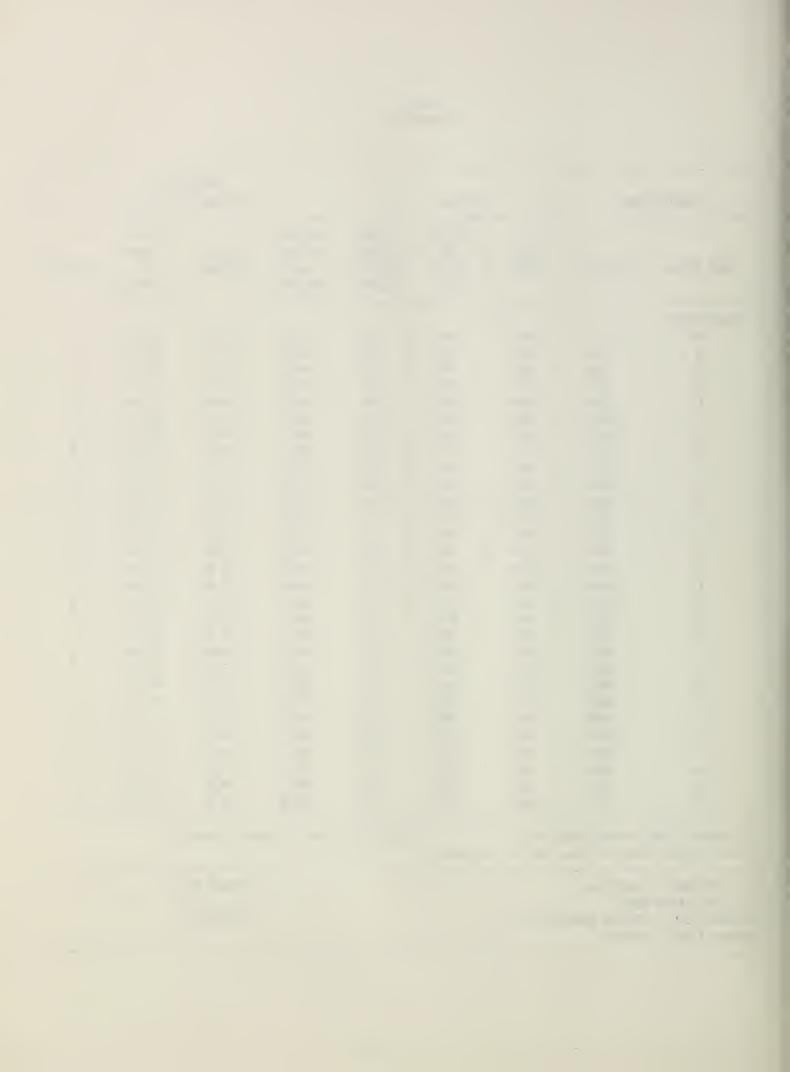
WILLOW CREEK

: SOIL CONSERVATION SERVICE

: WAUSHARA COUNTY, WISCONSIN

: WAUSHARA COUNTY FLOODPLAIN MANAGEMENT STUDY

E-2



Appendix F

INVESTIGATIONS AND ANALYSIS



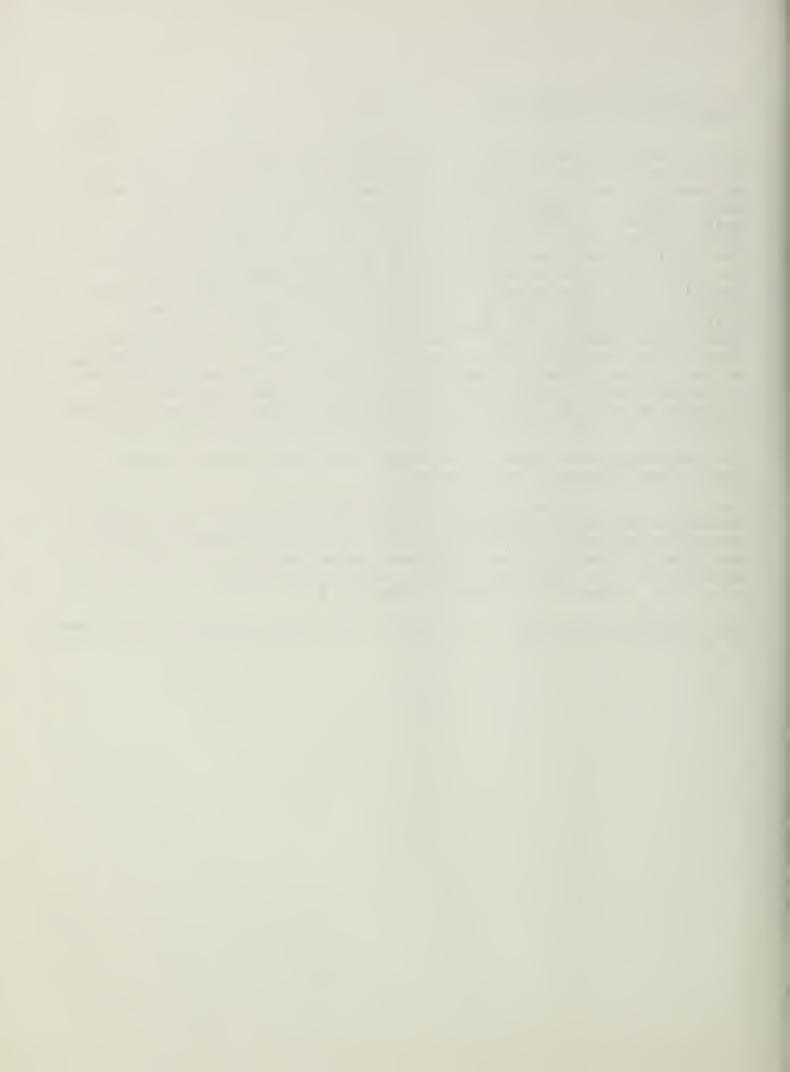
Investigation and Analysis

The Willow Creek watershed is relatively flat with many kettle depressions, swamps and lakes. The soils are sandy and rapidly permeable. Due to the nature of the watershed a single storm event model would not produce a reasonable floodflow. Using runoff only would not be realistic because it would be difficult to predict the amount of meltwater entering the soil. The 100-year March snowpack has eight inches of water equivalent (6). Should the snowpack melt rapidly a major flood could result. The USGS regression formula approach would give a more realistic floodflow (1). This approach is based on historical gage records of streams grouped in areas of similar topography. The gage records include the snowmelt and groundwater combined flows. The Auroraville dam is being replaced as part of the upgrading of Highway 49. The new structure was breached as part of the hydraulics analysis performed by Wisconsin Department of Transportation (DOT). The results of the breach analysis is included as part of this study. The hydraulic analysis was performed utilizing the Army Corp of Engineers hydrology program HEC-1 (9).

The resultant flows used in the study are listed for each cross section and frequency in Appendix E.

The flows and surveyed cross sections data were modeled in a step backwater computer program HEC-2 (10). The resultant water surface elevations for each cross section are listed in Appendix E and the profiles combined with the dam breach water surface are shown in Appendix B. The regulatory flood level is the higher of the 100-yr or the dam breach flood (Table 2 Appendix E).

The floodplain, floodway limits and cross section locations are shown on photomaps in Appendix A.



Appendix G

GLOSSARY



GLOSSARY

CHAPTER NR. 116, WISCONSIN'S FLOODPLAIN MANAGEMENT PROGRAM NR. 116.03 DEFINITIONS

Channel. A channel is a natural or artificial watercourse with definite bed and banks to confine and conduct the normal flow of water.

<u>Department</u>. Department refers to the State of Wisconsin Department of Natural Resources.

Encroachment. An encroachment is any fill, structure, building, use, accessory use, or development in the floodway.

Encroachment/Floodway Lines. Encroachment/floodway lines are limits of obstruction to floodflows. These lines are on both sides of and generally parallel to the river or stream. The lines are established by assuming that the area landward (outside) of the encroachment/floodway lines will be ultimately developed in such a way that it will not be available to convey floodflows.

<u>Flood</u>. A general and temporary condition of partial or complete inundation of normally dry land areas caused by the overflow or rise of rivers, streams, or lakes.

Flood Frequency. The term flood frequency is a means of expressing the probability of flood occurrences and is generally determined from statistical analyses. The frequency of a particular floodflow is usually expressed as occurring, on the average, once in a specified number of years. Any particular floodflow could, however, occur more frequently than once in any given year.

Flood Fringe. The flood fringe is that portion of the floodplain outside of the floodway, which is covered by floodwaters during the regional flood; it is generally associated with standing water rather than rapidly flowing water.

Floodplain. The floodplain is the land which has been or may be hereafter covered by floodwater during the regional flood. The floodplain includes the floodway and the flood fringe.

Floodplain Management. Floodplain management involves the full range of public policy and action for insuring wise use of floodplains. It includes everything from the collection and dissemination of flood control information to actual acquisition of floodplain lands; and the enactment and administration of codes, ordinances, and statutes for land use in the floodplain.

Flood Proofing. Flood proofing involves any combination of structural provisions, changes, or adjustments to properties and structures subject to flooding, primarily for the purpose of reducing or eliminating flood damage to properties, water and sanitary facilities, structures and contents of buildings in flood hazard areas.

Flood Protection Elevation. The flood protection elevation shall correspond to a point 2 feet of freeboard above the water surface profile associated with the regional flood and the official floodway lines. Also see: Freeboard.

Floodway. The floodway is the channel of a river or stream and those portions of the floodplain adjoining the channel required to carry and discharge the floodwater or floodflows associated with the regional flood.

Freeboard. Freeboard is a factor of safety usually expressed in terms of a certain amount of feet above a calculated flood level. Freeboard compensates for the many unknown factors that contribute to flood heights greater than the height calculated. These unknown factors include, but are not limited to, ice jams, debris accumulation, wave action, obstruction of bridge openings and floodways, the effects of urbanization on the hydrology of the watershed, loss of flood storage areas due to development and aggradation of the river or streambed.

High Flood Damage Potential. High flood damage potential is associated with any danger to life or health and any significant economic loss to a structure or building or its contents.

Hydraulic Floodway Lines. Hydraulic floodway lines shall delineate the channel of the river or stream and those portions of the adjoining floodplains which are reasonably required to carry and discharge the regional floodflow without any measurable increase in flood heights.

Hydraulic Reach. A hydraulic reach along a river or stream is that portion of the river or stream extending from one significant change in the hydraulic character of the river or stream to the next significant change. These changes are usually associated with breaks in the slope of the water surface profile, and may be caused by bridges, dams, expansion and contraction of the waterflow, and changes in streambed slope or vegetation.

Levee. A levee is a continuous dike or embankment of earth constructed parallel to a river or stream to prevent flooding of certain areas of land.

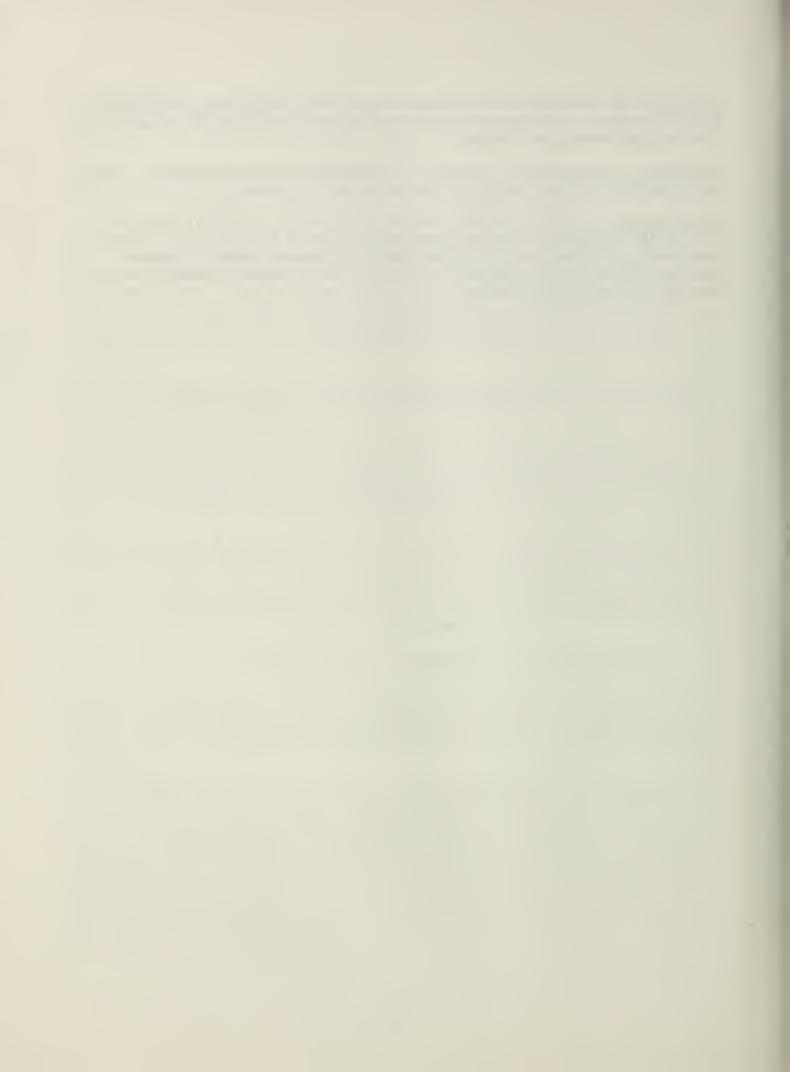
Official Floodway Lines. Official floodway lines are those lines which have been adopted by the county, city, or village, approved by the department, and which are shown on the official floodplain zoning maps and used for regulatory purposes.

Regional Flood. The regional flood is a flood determined to be representative of large floods known to have generally occurred in Wisconsin and which may be expected to occur on a particular stream because of like physical characteristics. The regional flood is based upon a statistical analysis of streamflow records available for the watershed and/or an analysis of rainfall and runoff characteristics in the general watershed region. The flood frequency of the regional flood is once in every 100 years; this means that in any given year there is a 1 percent chance that the regional flood may occur. During a typical 30-year mortgage period, the regional flood has a 26 percent chance of occurring.

Structure. A structure is any manmade object with form, shape, and utility, either permanently or temporarily attached to or placed upon the ground, riverbed, streambed, or lakebed.

Watershed. A watershed is a region or area contributing ultimately to the water supply of a particular watercourse or body of water.

<u>Water Surface Profile</u>. The water surface profile is a graphical representation of the height of the water surface throughout a county, city, or village based upon a certain flow passing through the river or stream. A water surface profile based upon flows occurring during a regional flood is used in regulating the floodplain areas.



Appendix H

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